

FOUNDATIONS (cont'd)

8.

EXCAVATION, BACKFILL, AND COMPACTION SHALL BE DONE IN STRICT ACCORDANCE WITH GEOTECHNICAL ENGINEERING INVESTIGATION REPORT RECOMMENDATIONS.
9.

FOUNDATION EARTHWORK SHALL BE OBSERVED BY A QUALIFIED GEOTECHNICAL ENGINEER, OR THEIR ASSIGNEE RETAINED BY OWNER AND SATISFACTORY TO ARCHITECT (STRUCTURAL ENGINEER) AND GOVERNING CODE AUTHORITY. PERFORM REQUIRED OBSERVATIONS OF THIS CONTRACT AND CBC SECTION 1705.6.
10.

FOUNDATION EXCAVATION, BACKFILLING, AND COMPACTION SHALL BE OBSERVED AND APPROVED BY A GEOTECHNICAL ENGINEER AND THE GOVERNING AGENCY PRIOR TO PLACING REINFORCING STEEL AND CONCRETE. GEOTECHNICAL ENGINEER SHALL PROVIDE A LETTER OF COMPLIANCE TO THE OWNER.
11.

TEMPORARY CUT SLOPES SHALL NOT EXCEED THOSE RECOMMENDED IN THE GEOTECHNICAL ENGINEERING INVESTIGATION REPORT. DO NOT PERMIT ANY PERSON TO DESCEND INTO TRENCHES OR EXCAVATIONS GREATER THAN FIVE FEET IN DEPTH UNLESS NECESSARY PERMIT FROM STATE OF CALIFORNIA DIVISION OF INDUSTRIAL SAFETY IS OBTAINED PRIOR TO ISSUANCE OF BUILDING OR GRADING PERMIT. CONTRACTOR TO PROVIDE FOR DESIGN, PERMIT, AND INSTALLATION OF ALL SHORING AND SHEATHING NECESSARY TO SAFELY RETAIN EARTH BANKS.
12.

CONTRACTOR TO PROVIDE FOR DEWATERING OF EXCAVATIONS FROM SURFACE WATER, GROUND WATER OR SEEPAGE. DEWATERING SHALL EFFECTIVELY ELIMINATE ANY HYDROSTATIC PRESSURE ON SHORING. ENSURE THAT CONTAMINATED WATER IS NOT DISPOSED OF IN PUBLIC SEWER OR STORM DRAIN SYSTEM AND ENSURE THAT DIRTY WATER IS NOT DISPOSED OF INTO PUBLIC RIGHT-OF-WAY.
13.

UNLESS ADEQUATELY BRACED AND SHORED, RETAINING WALLS SHALL NOT BE BACKFILLED UNTIL WALLS HAVE ATTAINED FULL DESIGN STRENGTH. FOR PIT WALLS AND BUILDING WALLS BELOW GRADE, BRACING AND SHORING SHALL REMAIN IN PLACE UNTIL ATTACHED FLOORS ARE PLACED, CURED FOR AT LEAST 7 DAYS, AND HAVE ATTAINED FULL DESIGN STRENGTH. BACKFILL PLACED IMMEDIATELY BEHIND RETAINING WALLS SHALL BE COMPACTED WITH HAND OPERATED EQUIPMENT.
14.

SIDEWALKS OR PAVING IMMEDIATELY ADJACENT TO BUILDING PERIMETER SHALL BE SLOPED TO PROVIDE POSITIVE DRAINAGE AWAY FROM BUILDING. LANDSCAPE IRRIGATION IS NOT PERMITTED WITHIN FIVE FEET OF BUILDING PERIMETER FOOTINGS EXCEPT WHEN ENCLOSED IN PROTECTED PLANTERS THAT DIRECT DRAINAGE AWAY FROM STRUCTURE AND FOUNDATIONS. DISCHARGE FROM DOWNSPOUTS, ROOF DRAINS AND SCUPPERS IS NOT PERMITTED ONTO UNPROTECTED SOILS WITHIN FIVE FEET OF BUILDING PERIMETER.
15.

COMPACTION REPORT SHALL BE SUBMITTED TO THE BUILDING INSPECTOR PRIOR TO FOOTING INSPECTION.

REINFORCING STEEL

1.

REINFORCING STEEL SHALL BE PLACED IN ACCORDANCE TO AMERICAN CONCRETE INSTITUTE ACI 318 "BUILDING CODE REQUIREMENTS FOR STRUCTURAL CONCRETE" AND CONCRETE REINFORCING STEEL INSTITUTE (CRSI) "MANUAL OF STANDARD PRACTICE".
2.

REINFORCING STEEL SHALL CONFORM TO ASTM A615/A615M, GRADE 60, UNLESS NOTE OTHERWISE. BARS TO BE WELDED SHALL CONFROM TO LOW ALLOY ASTM A706/A706M GRADE 60.
3.

DEFORMED LONGITUDINAL REINFORCEMENT RESISTING EARTHQUAKE-INDUCED MOMENT, AXIAL FORCE, OR BOTH, IN SPECIAL MOMENT FRAMES, SHALL COMPLY WITH ASTM A706/A706M, GRADE 60. ASTM A615/A615M GRADES 40 AND 60 REINFORCEMENT SHALL BE PERMITTED IN THESE MEMBERS IF:

A.

THE ACTUAL YIELD STRENGTH BASED ON MILL TESTS DOES NOT EXCEED THE SPECIFIED YIELD STRENGTH BY MORE THAN 18,000 PSI.

B.

FOR ASTM A615/A615M GRADE 40 REINFORCEMENT, THE RATIO OF THE ACTUAL TENSILE STRENGTH TO THE ACTUAL YIELD STRENGTH IS NOT LESS THAN 1.25.

C.

FOR ASTM A615/A615M GRADE 60 REINFORCEMENT, THE MINIMUM ELONGATION IN 8 INCHES SHALL BE AS FOLLOWS:

1.

NO. 3 THROUGH NO.6

= 14 PERCENT

2.

NO. 7 THROUGH NO. 11

= 12 PERCENT

3.

NO. 14 AND NO. 18

= 10 PERCENT

4.

WELDED WIRE REINFORCEMENT (WWR) SHALL CONFORM TO ASTM A1064. WELDED WIRE REINFORCEMENT LAP SPLICE LENGTH MEASURED BETWEEN OUTERMOST CROSS WIRE OF EACH REINFORCEMENT SHEET. SHALL BE PER SCHEDULE BELOW (8 INCHES MINIMUM).
- | WELDED WIRE SIZE | LAP SPLICE LENGTH IN INCHES<br>(ACI 318-14 § 25.5.3) |                  |                  |
|------------------|--|------------------|------------------|
|                  | 3000 PSI (LT WT)                                     | 4000 PSI (LT WT) | 5000 PSI (LT WT) |
| D4 - D6          | 12   | 12               | 12               |
| D7 - D9          | 16   | 14               | 13               |
| D10 - D12        | 24   | 21               | 19               |
5.

DEFORMED BAR ANCHORS SHALL BE NELSON STUD WELDING, INC. TYPE D2L (ICC EVALUATION SERVICE REPORT ESR-2907), OR AN APPROVED EQUAL, AND SHALL BE MADE FROM DEFORMED STEEL WIRE CONFORMING TO ASTM A1064, WITH A MINIMUM YIELD STRENGTH OF 70 KSI AND A MINIMUM TENSILE STRENGTH OF 80 KSI.

6.

LENTON FORM SAVER COUPLERS, FA OR FS SERIES, SHALL BE BY ERICO INTERNATIONAL CORPORATION (IAPMO UES EVALUATION REPORT NO. 0129) OR APPROVED EQUAL.

7.

HEADED DEFORMED BARS SHALL BE LENTON TERMINATORS BY ERICO INTERNATIONAL CORPORATION (IAPMO UES EVALUATION REPORT NO. 0188) OR APPROVED EQUAL. HEADED DEFORMED BARS MAY BE USED IN LIEU OF STANDARD HOOKS ONLY WHERE SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER.

8.

MECHANICAL COUPLERS FOR SPLICING REINFORCING BARS SHALL BE LENTON STANDARD COUPLERS, LENTON TRANSITION COUPLERS, SA OR FS SERIES, BY ERICO INTERNATIONAL CORPORATION (IAPMO UES EVALUATION REPORT NO. 0129) OR APPROVED EQUAL. MECHANICAL COUPLERS MAY BE USED IN LIEU OF LAP SPLICING REINFORCING BARS ONLY WHERE SPECIFICALLY APPROVED BY THE STRUCTURAL ENGINEER.

9.

PREPARE REINFORCING STEEL SHOP DRAWINGS IN ACCORDANCE TO ACI 315, PART B. SHOP DRAWINGS MAY BE PREPARED MANUALLY OR BY COMPUTER. PLACING DRAWINGS SHALL BE PREPARED TO THE SAME STANDARD AS CONTRACT DRAWINGS. SHOW REINFORCING PLACEMENT, SPLICE LOCATIONS, REINFORCING LENGTHS, DETAILS, ELEVATIONS, BEND DETAILS, ETC. SUBMIT TO ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW PRIOR TO FABRICATION. PROMPTLY NOTIFY ARCHITECT (STRUCTURAL ENGINEER) PRIOR TO DEVELOPING REINFORCING STEEL SHOP DRAWINGS IF INSUFFICIENT CLEAR DISTANCES BETWEEN REINFORCING STEEL OR OTHER CONGESTION IS ENCOUNTERED. DEVIATIONS FROM THE CONTRACT DOCUMENTS SHALL BE CLEARLY IDENTIFIED ON THE SHOP DRAWINGS. IF SUBMITTAL IS PARTIAL, CLEARLY INDICATE ITEMS EXCLUDED FROM SUBMITTAL. SHOP DRAWINGS WILL BE REJECTED IF NOT PREPARED TO THE STANDARDS STATED ABOVE.
- GENERAL (cont'd)
14.

EQUIPMENT MANUFACTURER SHALL PROVIDE EQUIPMENT ANCHORAGE TO THE STRUCTURE MEETING THE REQUIREMENTS OF ASCE/SEI 7, CHAPTER 13.6. USE ISOLATORS, FASTENERS AND BRACING HAVING CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT. EQUIPMENT ANCHORAGE SHALL BE CAPABLE OF TRANSMITTING CODE REQUIRED LATERAL LOADS BUT IN NO EVENT LESS THAN LATERAL LOAD EQUIVALENT TO 50 PERCENT OF THE OPERATING WEIGHT OF EQUIPMENT. SECURE SUSPENDED EQUIPMENT WITH LATERAL OR SWAY BRACING HAVING CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT

15.

PIPING AND DUCTWORK BRACING SHALL BE IN ACCORDANCE WITH THE LATEST EDITION OF THE SHEET METAL AND AIR CONDITIONING CONTRACTORS' NATIONAL ASSOCIATION (SMACNA) "SEISMIC RESTRAINT MANUAL - GUIDELINES FOR MECHANICAL SYSTEMS", INCLUDING ADDENDA.

16.

"TYPICAL DETAILS" ARE APPLICABLE THROUGHOUT CONSTRUCTION DOCUMENTS AND MAY NOT BE SPECIFICALLY REFERENCED THEREIN. CONTRACTOR IS RESPONSIBLE FOR IDENTIFYING THESE TYPICAL DETAILS AND UNDERSTANDING EXTENT OF THEIR APPLICATION PRIOR TO PERFORMING WORK.

17.

UNLESS SPECIFICALLY SHOWN ON THE PLANS NO STRUCTURAL MEMBER SHALL BE CUT, DRILLED OR NOTCHED WITHOUT PRIOR WRITTEN AUTHORIZATION FROM THE STRUCTURAL ENGINEER REVIEW AND APPROVAL.

18.

UNLESS NOTED ON STRUCTURAL DRAWINGS SEE ARCHITECTURAL DRAWINGS FOR INFORMATION NOTED BELOW :

A.

SIZE AND LOCATION OF DOOR AND WINDOW OPENINGS IN STRUCTURAL WALLS

B.

SIZE AND LOCATION OF FLOOR AND ROOF OPENINGS AND SLAB EDGES

C.

SIZE AND LOCATION OF NON-BEARING CMU WALLS AND OPENINGS THEREIN

D.

SIZE AND LOCATION OF CONCRETE CURBS, SLOPES, DEPRESSIONS, CHANGES IN LEVEL, CHAMFERS AND REVEALS, INSERTS FOR FINISH SYSTEMS

E.

EXTERIOR WALL SYSTEM AND LOCATION

F.

STAIR SIZE AND LOCATION, FRAMING AND DETAILS

G.

DIMENSIONS NOT SHOWN ON STRUCTURAL DRAWINGS

H.

ALL HOLES IN STRUCT WALLS NOT SPECIFICALLY SHOWN AND IDENTIFIED ON THE STRUCT DWGS. AND AFFECTING THE STRUCTURAL INTEGRITY OF THE WALL SHALL BE BROUGHT TO THE ATTENTION OF THE S.E.O.R. & FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION

19.

SEE MECHANICAL, ELECTRICAL, PLUMBING DRAWINGS FOR INFORMATION NOTED BELOW:

A.

SIZE AND LOCATION OF EQUIPMENT PADS, EQUIPMENT ANCHORAGE TO STRUCTURE, AND EQUIPMENT WEIGHTS

B.

ANCHORAGE OF DUCTWORK, PIPING, ELECTRICAL CONDUITS TO STRUCTURE

C.

ELECTRICAL CONDUIT RUNS, OUTLETS AND BOXES IN CONCRETE SLABS AND WALLS

D.

PIPE SLEEVES, TRENCHES, AND OPENINGS THROUGH WALLS AND SLABS FOR DUCTWORK, PIPE RUNS, ELECTRICAL CONDUIT RUNS

E.

ALL HOLES IN STRUCT WALLS NOT SPECIFICALLY SHOWN AND IDENTIFIED ON THE STRUCT DWGS. AND AFFECTING THE STRUCTURAL INTEGRITY OF THE WALL SHALL BE BROUGHT TO THE ATTENTION OF THE S.E.O.R. FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.

20.

MECHANICAL, ELECTRICAL AND PLUMBING LOADS SHALL BE SUPPORTED FROM BEAMS. EXCEPTION: LIGHT MECHANICAL, ELECTRICAL AND PLUMBING LOADS MAYBE SUPPORTED BY CONCRETE-ON-METAL DECK ASSEMBLY, BUT MUST BE ANCHORED INTO STRUCTURAL CONCRETE SYSTEM BY A SYSTEM HAVING CURRENT ICC-ES OR IAPMO UES EVALUATION REPORT.

21.

NON-STRUCTURAL ITEMS, INCLUDING BUT NOT LIMITED TO, STAIR FRAMING, ARCHITECTURAL CLADDING, ETC., WHEN NOT DETAILED ON THE STRUCTURAL OR ARCHITECTURAL DRAWINGS, SHALL BE THE DESIGN RESPONSIBILITY OF THE CONTRACTOR. THESE NON-STRUCTURAL ITEMS MAY BE SUPPORTED BY THE PRIMARY STRUCTURE, BUT SHALL NOT IMPOSE TORSIONAL LOADS ONTO THE PRIMARY SUPPORT MEMBERS. PROVIDE BRACES, KICKERS, STIFFENERS, ETC., AS NECESSARY TO ELIMINATE TORSIONAL LOADS AT NO ADDITIONAL COSTS TO THE OWNER.
- FOUNDATIONS
1.

DESIGN OF FOUNDATION SYSTEM BASED ON RECOMMENDATIONS IN GEOTECHNICAL ENGINEERING INVESTIGATION REPORT BY ENGEO, INC. PROJECT NO. 6538.100312, DATED MAY 14, 2025, AND ALL SUBSEQUENT ADDENDA. GEOTECHNICAL REPORT AND ADDENDA SHALL BE CONSIDERED PART OF THESE CONTRACT DOCUMENTS AND SHALL BE KEPT AT JOB SITE AT ALL TIMES.

2.

FOUNDATION SYSTEM FOR THE STRUCTURE SHALL BE SPREAD FOOTING AND CONTINUOUS FOOTING.

3.

ISOLATED SPREAD FOOTING AND CONTINUOUS FOOTING DESIGN BASED ON ALLOWABLE NET BEARING PRESSURES OF 3000 PSF. BOTTOM OF ALL FOOTINGS SHALL BE A MINIMUM OF 18 INCHES BELOW LOWEST ADJACENT FLOOR OR GRADE. FOOTING DIMENSIONS SHALL NOT BE LESS THAN 18 INCHES FOR ISOLATED SPREAD FOOTINGS, AND 12 INCHES FOR CONTINUOUS STRIP FOOTINGS. ALLOWABLE BEARING PRESSURES CAN BE INCREASED 33 PERCENT FOR SEISMIC OR WIND LOADING.

4.

RESISTANCE TO LATERAL LOADS PROVIDED BY EITHER FRICTION AGAINST BASE OR BY PASSIVE EARTH PRESSURES. ALLOWABLE COEFFICIENT OF FRICTION IS 0.35 AND ALLOWABLE PASSIVE PRESSURE OF 350 PCF. WHERE BOTH THE FRICTION AND THE PASSIVE RESISTANCE ARE UTILIZED FOR SLIDING RESISTANCE. A ONE THIRD INCREASE IN THE FRICTION AND PASSIVE RESISTANCE VALUE MAY BE USED FOR WIND OR SEISMIC LOADS.

5.

LIGHT POLE FOUNDATION MAY BE DESIGNED TO IMPOSE AN ALLOWABLE LATERAL BEARING PRASSURE OF 350 PSF PER FOOT BELOW GRADE. LATERAL BEARING PREASSURE MAY BE INCREASED BY ONE-THIRD FOR SHORT-DURATION LOADING SUCH AS WIND OR SEISMIC LOADING.

6.

FOUNDATIONS MAY BE CAST DIRECTLY AGAINST EXCAVATIONS PROVIDED EXCAVATION IS CAPABLE OF MAINTAINING A VERTICAL CUT WITHOUT SLOUGHING. FOUNDATION DIMENSION SHALL BE ENLARGED BY AN ADDITIONAL ONE INCH IN THE DIRECTION OF THE SIDE CAST AGAINST EARTH.

7.

CONCRETE SHALL NOT BE PLACED ON FROZEN GRADE. IF FOOTING IS SUBJECT TO FREEZING TEMPERATURE AFTER FOUNDATION CONSTRUCTION, THEN FOOTING SHALL BE ADEQUATELY PROTECTED FROM FREEZING.
- GENERAL
1.

ALL WORK SHALL CONFORM TO THE STANDARDS OF LOS ANGELES COUNTY BUILDING CODE, 2023 EDITION, AND THOSE CODES AND STANDARDS LISTED IN THE CONTRACT DOCUMENTS.

2.

THE PROJECT MANUAL FORMS A PART OF THESE GENERAL NOTES. CODES, STANDARDS, AND SPECIFICATIONS, INCLUDING ADDENDA AND SUPPLEMENTS, REFERENCED IN THE CONTRACT DOCUMENTS SHALL BE THE LATEST APPROVED ISSUE, UNLESS SPECIFICALLY NOTED.

3.

NOTES AND DETAILS ON DRAWINGS TAKE PRECEDENCE OVER GENERAL NOTES AND TYPICAL DETAILS. IF CONFLICT OCCURS BETWEEN THE CONTRACT DRAWINGS AND THE PROJECT MANUAL, IMMEDIATELY NOTIFY ARCHITECT (STRUCTURAL ENGINEER) FOR RESOLUTION. DIMENSIONS TAKE PRECEDENCE OVER SCALED DRAWINGS.

4.

DESIGN LOADS : REFER TO LOAD MAP PLANS, SHEETS S110.

DESIGN LIVE LOADS ARE NOTED AS FOLLOWS:

ROOF

= 20 PSF

MECHANICAL AREAS

= 50PSF (UNREDUCED)

5.

CODE LEVEL WIND DESIGN DATA:

BASIC WIND SPEED

= 105 MPH

EXPOSURE CATEGORY

= C

ENCLOSURE CLASSIFICATION

= ENCLOSED BUILDING

INTERNAL PRESSURE COEFFICIENT, GCpi

= ± 0.18

COMPONENTS AND CLADDING WIND PRESSURES, ASD (W/1.6).

	EFFECTIVE AREA	GENERAL (ZONE 4)	CORNER (ZONE 5)
WALL SURFACE	50 SF 100 SF	15.6 PSF 15.0 PSF	18.5 PSF 16.6 PSF
6.

CODE LEVEL EARTHQUAKE DESIGN DATA:

SITE COORDINATES

= 34.4143°N, 118.6020°W

MAPPED SPECTRAL RESPONSE ACCELERATION, SS

= 2.263g

MAPPED SPECTRAL RESPONSE ACCELERATION, S1

= 0.817g

SITE CLASS

= C

DESIGN SPECTRAL RESPONSE COEFFICIENT, SDS

= 1.601g

DESIGN SPECTRAL RESPONSE COEFFICIENT, SD1

= 0.710g

RISK CATEGORY

= IV

IMPORTANCE FACTOR, I<sub>e</sub>

= 1.5

SEISMIC DESIGN CATEGORY

= F

SEISMIC RESPONSE COEFFICIENT, CS

= 0.369

ANALYTICAL PROCEDURE:

=EQUIVALENT LATERAL FORCE PROCEDURE

SEISMIC-FORCE RESISTING SYSTEM

= LIGHT FRAME BEARING WOOD PANEL SHEAR WALLS

RESPONSE MODIFICATION FACTOR, R

= 6 1/2

DEFLECTION AMPLIFICATION FACTOR, Cd

= 4

OVERSTRENGTH FACTOR, Ω<sub>o</sub>

= 3

DESIGN BASE SHEAR

MAIN BUILDING

= 154 KIP

RESERVE APPARATUS

= 26.5 KIP
7.

GOVERNING CODE AUTHORITY: COUNTY OF LOS ANGELES
8.

CONTRACT DOCUMENTS INDICATE INFORMATION SUFFICIENT TO CONVEY DESIGN INTENT. REVIEW CONTRACT DOCUMENTS AND VERIFY FIELD AND EXISTING CONDITIONS. PROMPTLY NOTIFY ARCHITECT (STRUCTURAL ENGINEER), PRIOR TO PROCEEDING WITH WORK, IF FURTHER CLARIFICATION OF DESIGN INTENT IS NEEDED.
9.

VERIFY ALL DIMENSIONS PRIOR TO CONSTRUCTION AND NOTIFY THE ARCHITECT (STRUCTURAL ENGINEER) OF ANY DISCREPANCIES.
10.

PERFORM STRUCTURAL RELATED WORK AND DEVELOP SHOP DRAWINGS CONSIDERING CONTRACT DOCUMENTS IN THEIR ENTIRETY. CONDITIONS NOT SPECIFICALLY DETAILED SHALL BE CONSTRUCTED AS DETAILED FOR SIMILAR WORK.
11.

CONTRACT DOCUMENTS REPRESENT THE FINISHED STRUCTURE AND DO NOT INDICATE THE MEANS AND METHODS OF CONSTRUCTION. PROVIDE ALL NECESSARY MEASURES TO PROTECT THE STRUCTURE DURING CONSTRUCTION. COMPLY WITH THE STATE OF CALIFORNIA, DIVISION OF OCCUPATIONAL SAFETY AND HEALTH REGULATIONS. CONSTRUCTION MATERIALS, IF PLACED ON FRAMED FLOORS AND ROOFS, SHALL BE SPREAD OUT SUCH THAT THE DESIGN LIVE LOAD PER SQUARE FOOT IS NOT EXCEEDED. PROVIDE ADEQUATE SHORING IF OVERLOAD IS ANTICIPATED OR WHERE STRUCTURAL ELEMENTS HAVE NOT ATTAINED DESIGN STRENGTH. OBSERVATION VISITS TO THE SITE BY THE STRUCTURAL ENGINEER SHALL NOT CONSTITUTE ACCEPTANCE OF CONSTRUCTION MEANS AND METHODS.
12.

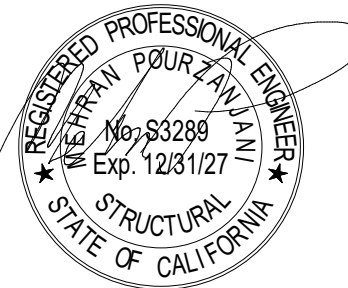
SUBMIT SHOP DRAWINGS FOR REVIEW BEFORE FABRICATION. CONTRACTOR SHALL REVIEW FOR COMPLETENESS AND COMPLIANCE WITH CONTRACT DOCUMENTS PRIOR TO SUBMISSION TO ARCHITECT (STRUCTURAL ENGINEER). ARCHITECT'S (STRUCTURAL ENGINEER'S) REVIEW IS FOR GENERAL CONFORMANCE WITH DESIGN INTENT AND DOES NOT CONSTITUTE AN AUTHORIZATION TO DEVIATE FROM TERMS AND CONDITIONS OF CONTRACT. WHEN INDICATED, THE SUBMITTAL SHALL BE SIGNED AND SEALED BY A PROFESSIONAL CIVIL OR STRUCTURAL ENGINEER LICENSED IN THE STATE OF CALIFORNIA. MAINTAIN AT SITE A COPY OF REVIEWED AND ACCEPTED SUBMITTALS.
13.

MODIFICATIONS AND SUBSTITUTIONS MUST BE ACCEPTED IN WRITING BY ARCHITECT (STRUCTURAL ENGINEER). NO MODIFICATION OR SUBSTITUTION WILL BE ACCEPTED VIA SHOP DRAWING REVIEW. MANUFACTURED MATERIALS SHALL BE APPROVED BY THE GOVERNING CODE AUTHORITY PRIOR TO THEIR USE. ADHERE TO ALL CONDITIONS OF THOSE APPROVALS. MODIFICATIONS AND SUBSTITUTIONS AFFECTING STRUCTURAL SAFETY, FIRE LIFE SAFETY OR ACCESSIBILITY ASPECTS OF THE PROJECT SHALL BE SUBMITTED TO THE ARCHITECT (STRUCTURAL ENGINEER) FOR REVIEW AND APPROVAL PRIOR TO FABRICATION/CONSTRUCTION.

GENERAL NOTES

FIRE STATION 46

MISSION VILLAGE  
COUNTY OF LOS ANGELES FIRE DEPARTMENT  
VALENCIA, CALIFORNIA



THE ABOVE DRAWINGS AND SPECIFICATIONS AND FINAL DESIGNS AND ARRANGEMENTS REPRESENTED THEREBY ARE AND SHALL REMAIN THE PROPERTY OF THE ARCHITECT, AND NO PART THEREOF SHALL BE COPIED, DISCLOSED TO OTHERS OR USED IN CONNECTION WITH ANY WORK OR PROJECT OTHER THAN THE SPECIFIC PROJECT FOR WHICH THEY HAVE BEEN PREPARED, AND DEVELOPED WITHOUT THE WRITTEN CONSENT OF THE ARCHITECT. VERBAL CONTACT WITH THESE DRAWINGS OR SPECIFICATIONS SHALL CONSTITUTE CONCLUSIVE EVIDENCE OF ACCEPTANCE OF THE INSTRUCTIONS.

WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER SCALED DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB, AND THE OFFICE MUST BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS SHOWN IN THESE DRAWINGS. SHOP DETAILS MUST BE SUBMITTED TO THIS OFFICE FOR APPROVAL, BEFORE PROCEEDING WITH FABRICATION.

10/20/2025

Date	Issue Date
Drawn	
Checked	
Scale	AS NOTED
Job. No.	Project Number

S001

WILLIAM LOYD JONES  
ARCHITECT

9415 culver boulevard  
culver city, california  
9 0 2 3 2

TEL 310 392 3995



SCALE: NTS

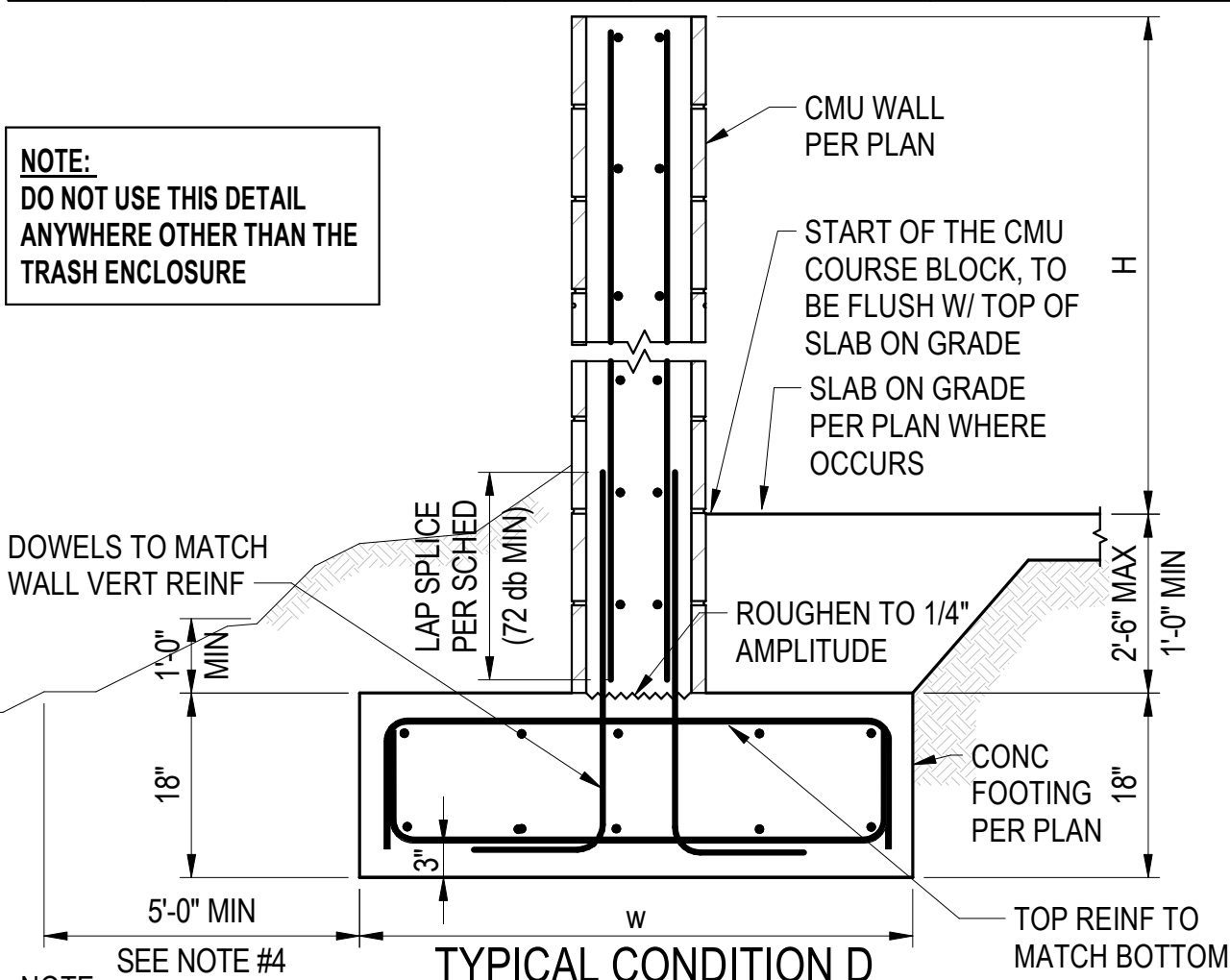


REV 01/06/2

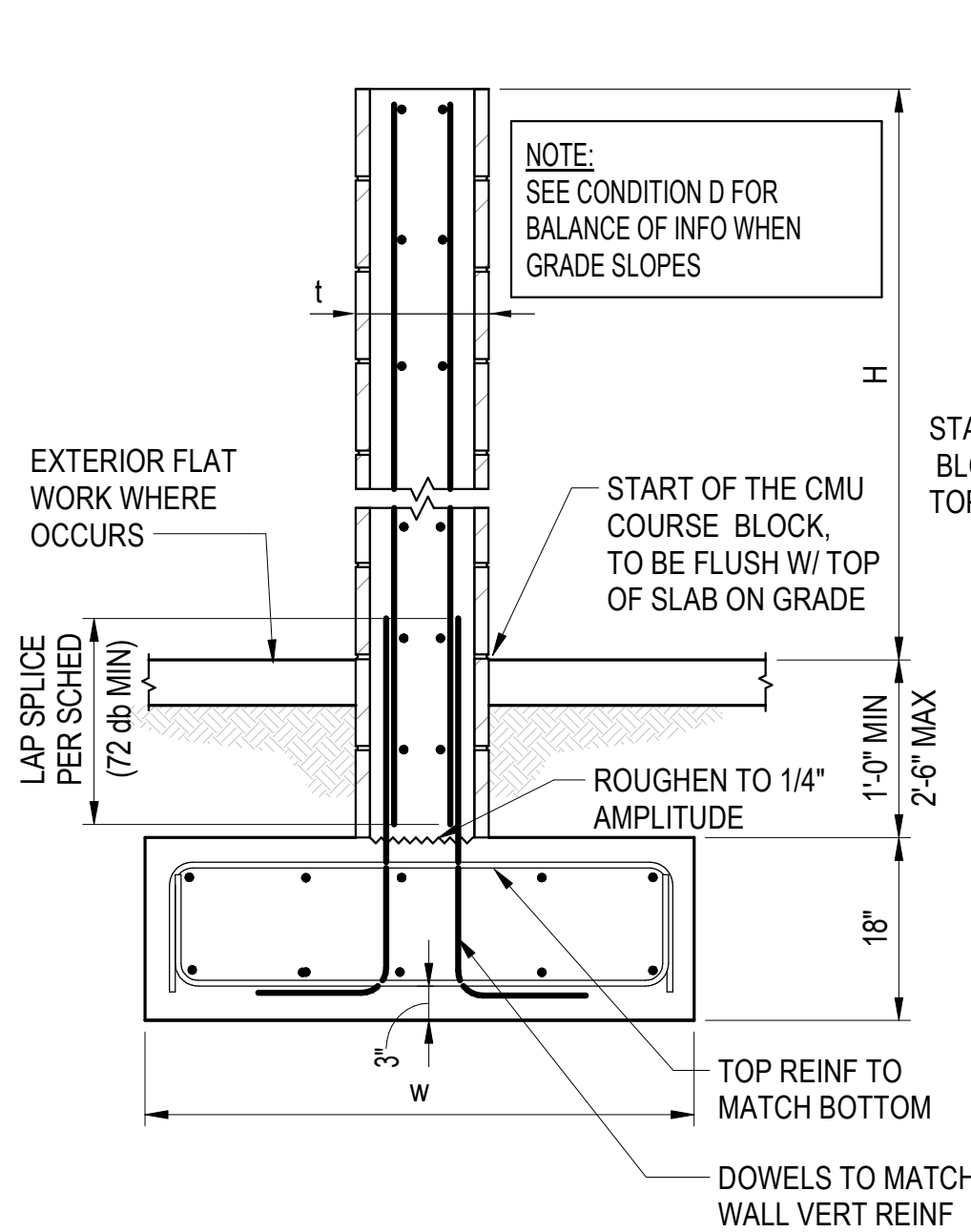
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PI AN CHECK SI I BMITTAI	- October 31 2025
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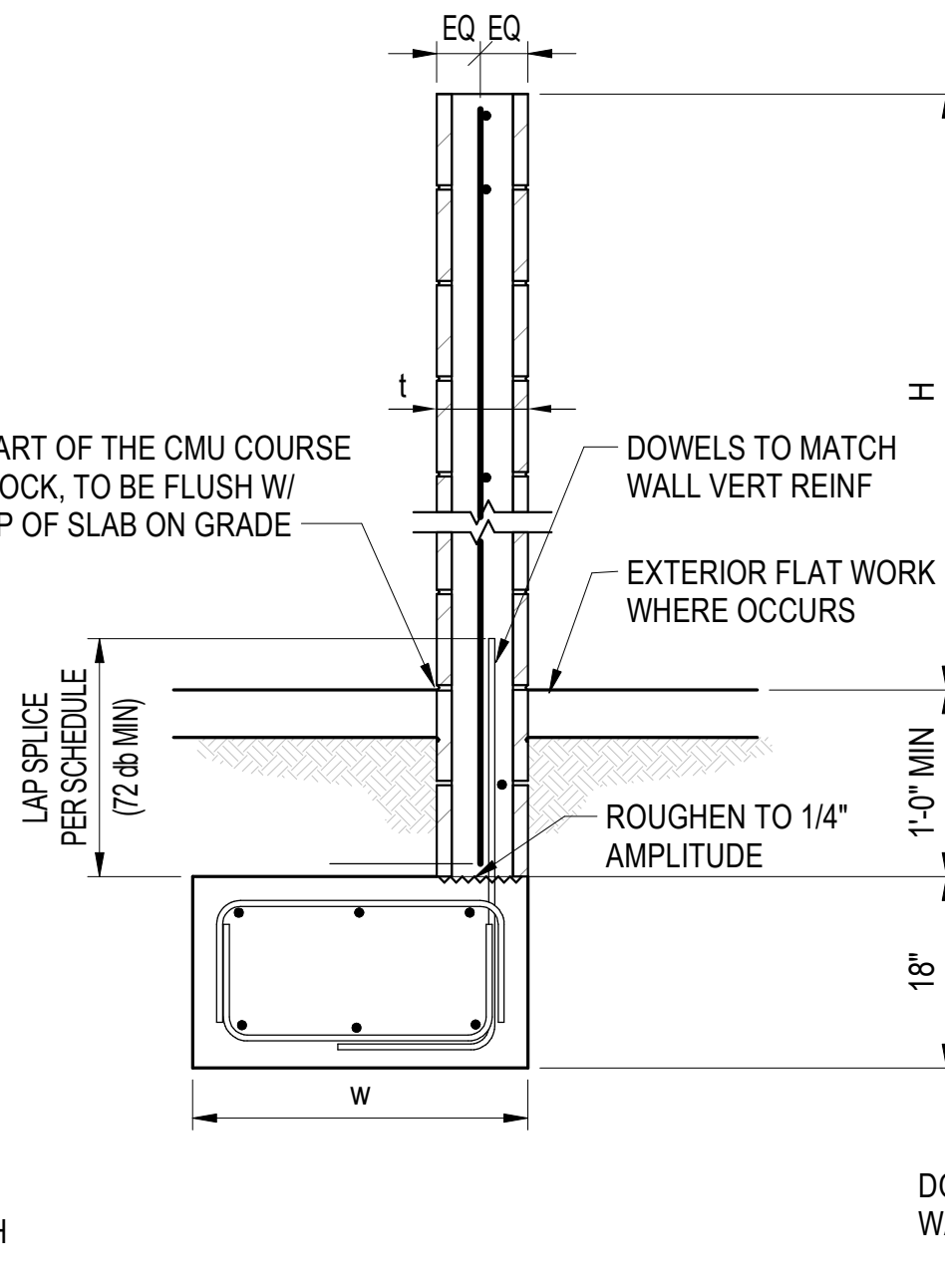
SCHEDULE FOR CONDITION A, B, AND C					
MAX H	WALL		FOOTING		REMARKS
	t	REINFORCING	w	REINFORCING	
4'-6"	8"	#4(V)@16" #4(H)@16"	3'-6"	(3)-#4(T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION A f <sub>m</sub> = 1500 PSI
4'-6"	8"	#4(V)@16" #4(H)@16"	3'-6"	(4)-#4(T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION B f <sub>m</sub> = 1500 PSI
7'-0"	8"	#5(V)@16" #4(H)@16"	4'-6"	(5)-#5 (T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION A f <sub>m</sub> = 2500 PSI
9'-0"	8"	#5(V)@16" EF #4(H)@16" EF	5'-6"	(6)-#5 (T&B) LONG #6(T&B)@12" SHORT	SEE CONDITION D & C f <sub>m</sub> = 2500 PSI
12'-0"	12"	#5(V)@8" EF #4(H)@16" EF	8'-0"	(7)-#5 (T&B) LONG #6(T&B)@9" SHORT	SEE CONDITION D & C f <sub>m</sub> = 2500 PSI



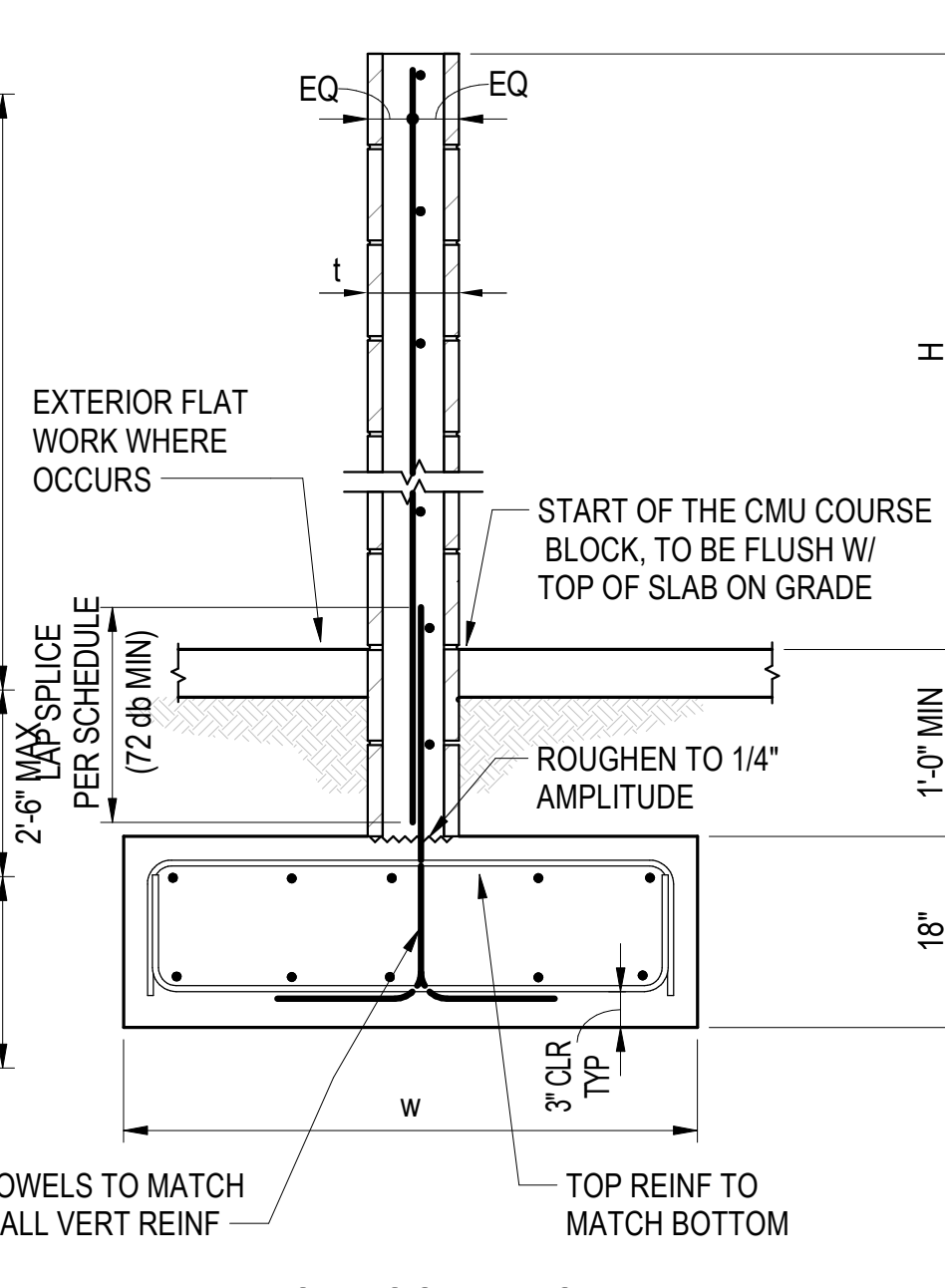
- NOTE:
- WHERE MASONRY WALL FOOTING TIES INTO COLUMN FOOTING, EXTEND MASONRY WALL FOOTING REINFORCING TO THE FAR END OF THE COLUMN FOOTING AND HOOK REINFORCING ENDS.
  - WHERE MASONRY WALL IS RETAINING SOIL ON ONE SIDE, THE MASONRY WALL SHALL HAVE DRAINAGE PER THE GEOTECHNICAL RECOMMENDATIONS AND CIVIL DRAWINGS TO ENSURE A DRAINAGE BACKFILL CONDITION (E.G. NO HYDROSTATIC PRESSURE ON THE MASONRY WALL).
  - WHERE WALL "H" IS BETWEEN VALUES INDICATED, USE DESIGN VALUES FOR HIGHER LISTED "H".
  - LOWER THE FOOTING AS REQUIRED TO ACHIEVE MINIMUM DIMENSIONS INDICATED AND SHOWN



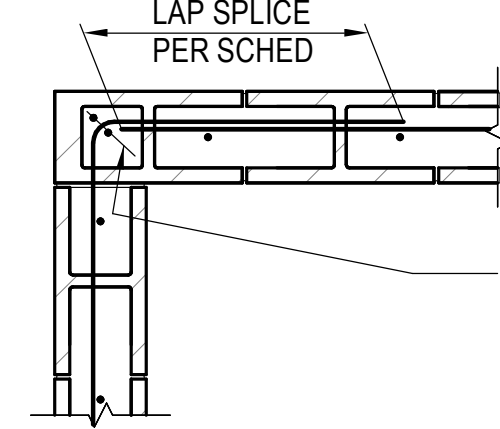
TYPICAL CONDITION C



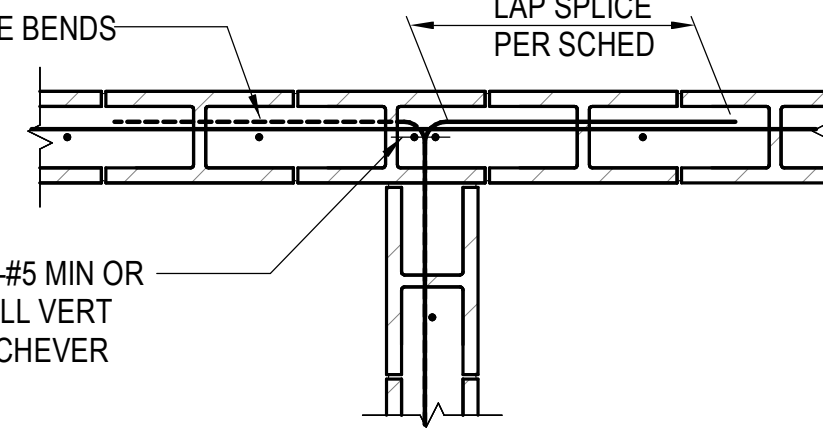
TYPICAL CONDITION B  
AT PROPERTY LINE



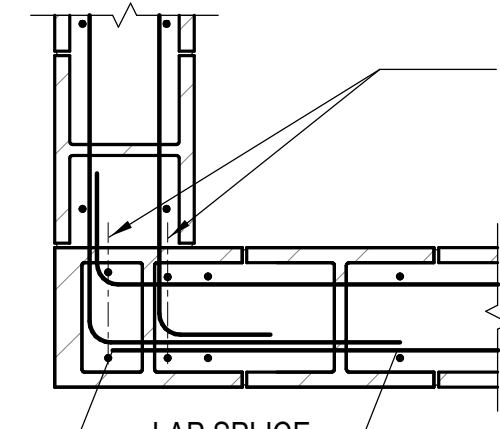
TYPICAL CONDITION A



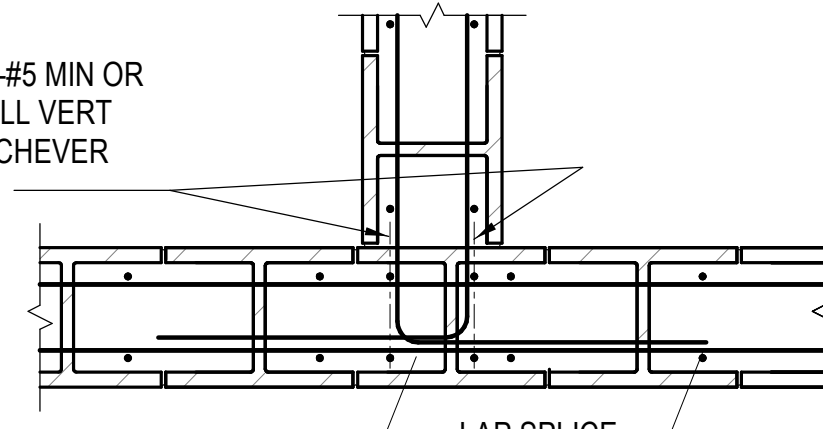
CORNER (SINGLE CURTAIN)



INTERSECTION (SINGLE CURTAIN)



CORNER (DOUBLE CURTAIN)

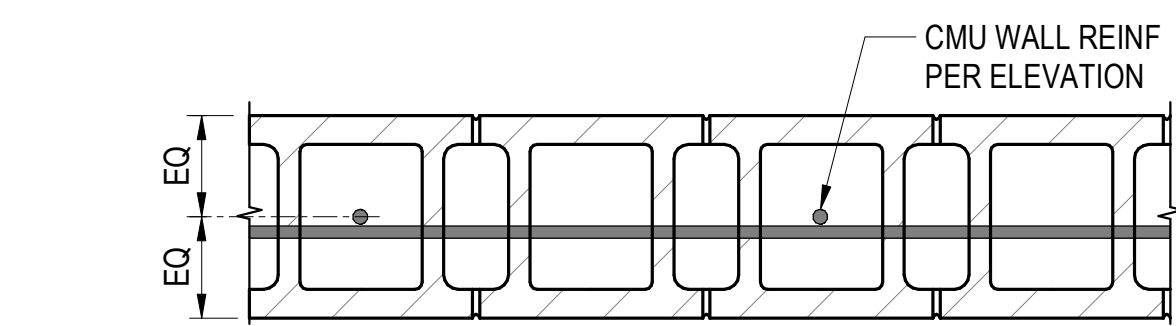


INTERSECTION (DOUBLE CURTAIN)

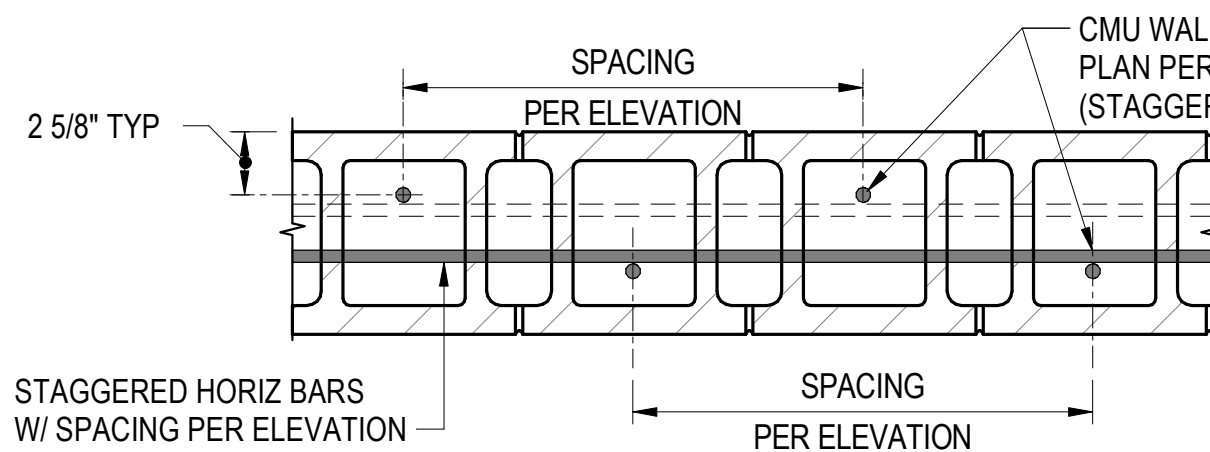
TYPICAL MASONRY WALL AT INTERSECTIONS DETAIL

SCALE: NTS  
REV 01/06/21

3



8" CMU WALL - REBAR CENTERED



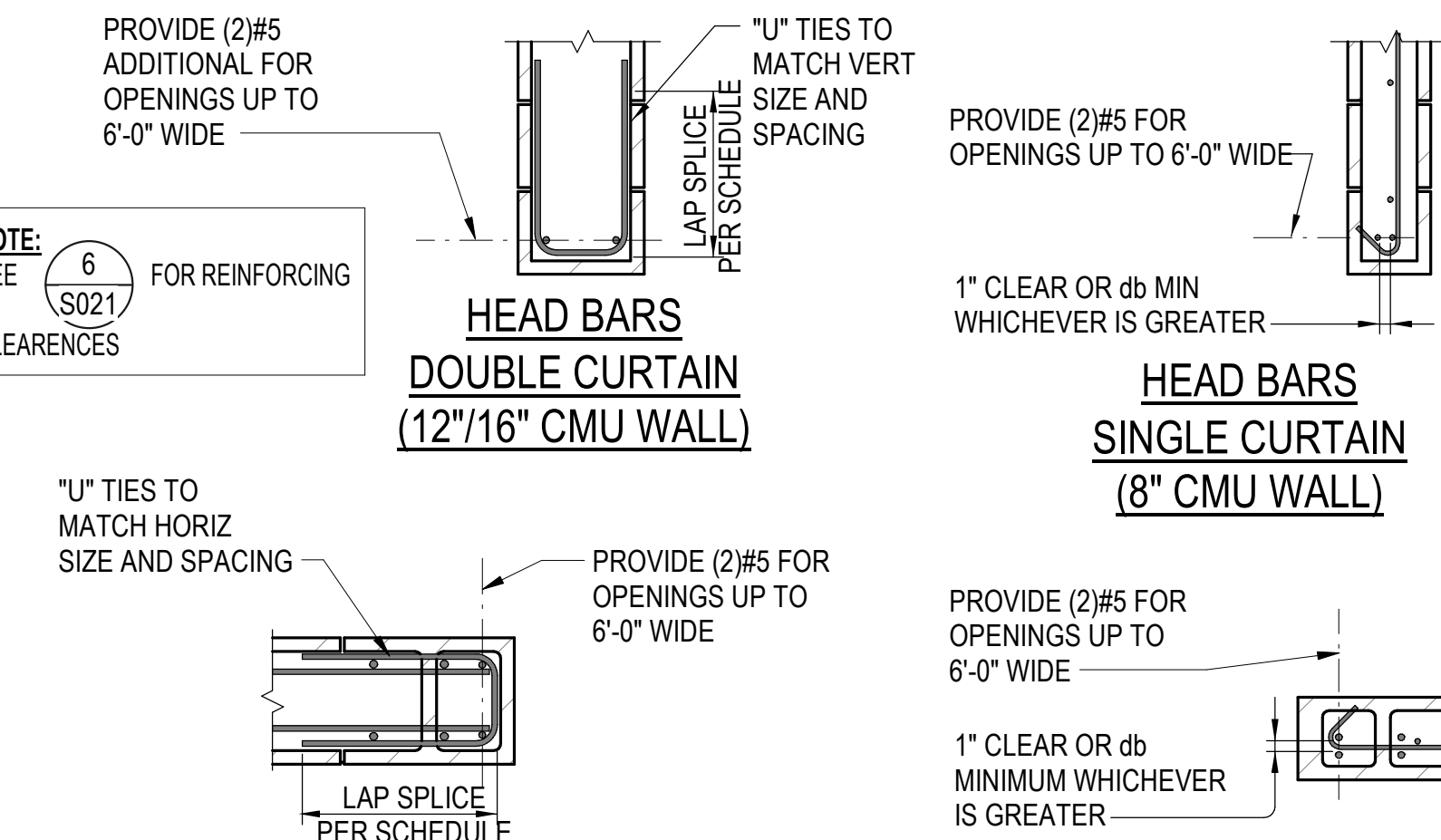
8" CMU WALL - REBAR EACH FACE STAGGERED

- NOTE:
- REFER TO S021 FOR BALANCE OF INFO.

TYPICAL CMU WALL REINFORCING PLACEMENT DETAIL

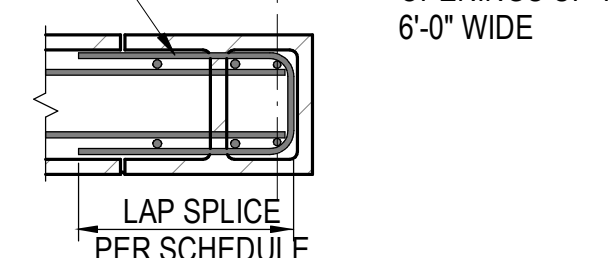
SCALE: NTS

6

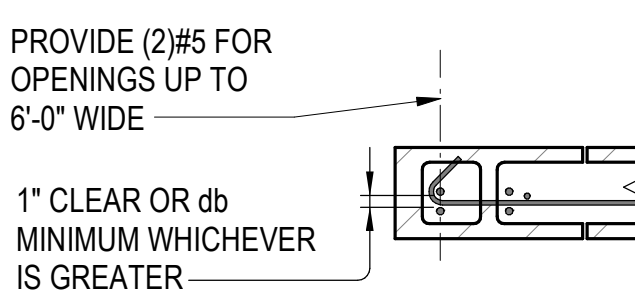


HEAD BARS  
DOUBLE CURTAIN  
(12"/16" CMU WALL)

HEAD BARS  
SINGLE CURTAIN  
(8" CMU WALL)



END OF WALL OR JAMB BARS  
DOUBLE CURTAIN  
(12"/16" CMU WALL)

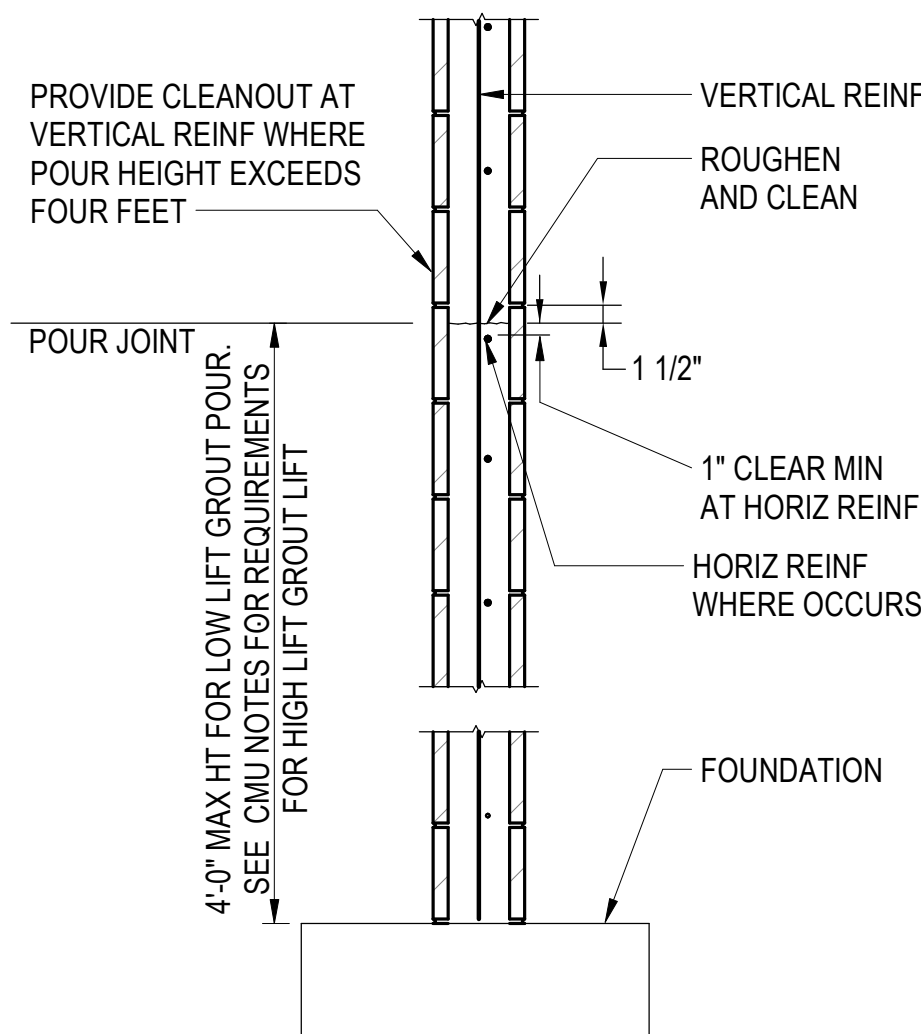


END OF WALL OR JAMB BARS  
SINGLE CURTAIN  
(8" CMU WALL)

TYPICAL MASONRY WALL AT OPENINGS AND WALL ENDS DETAIL

SCALE: NTS

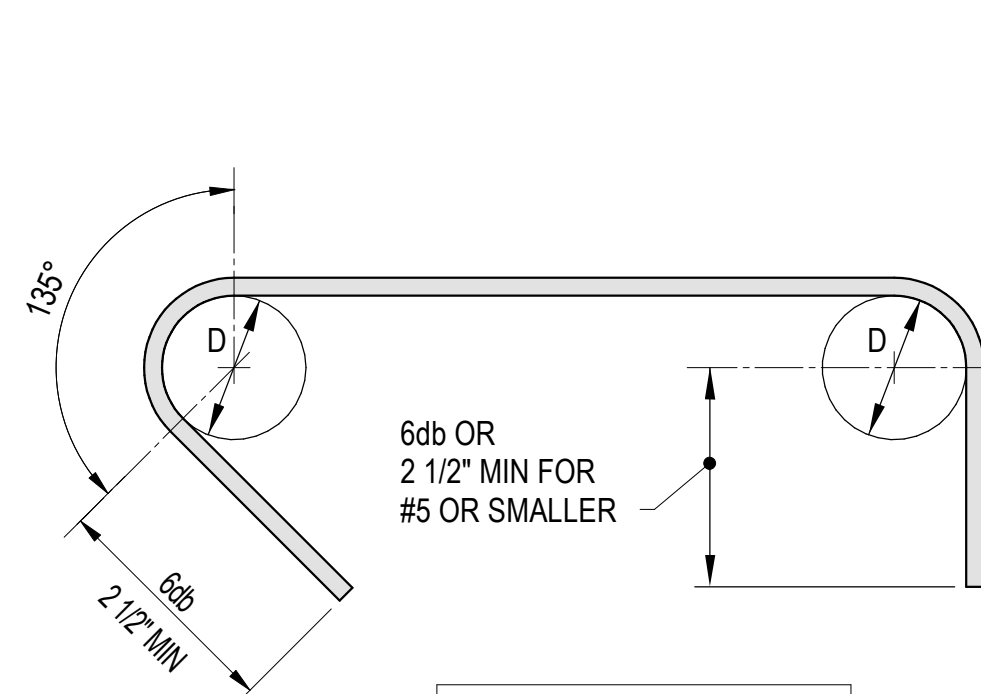
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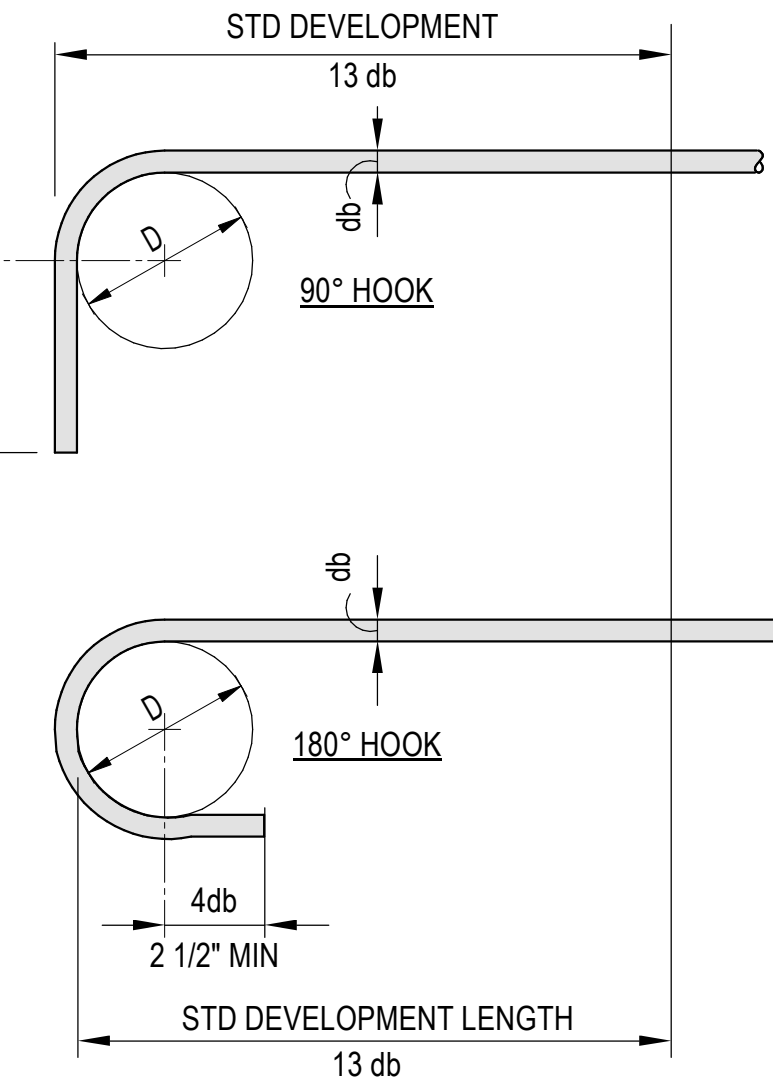
TYPICAL HORIZONTAL CONSTRUCTION  
JOINT IN MASONRY WALL DETAIL

SCALE: NTS

4



STANDARD HOOKS FOR TIES AND STIRRUPS



TYPICAL STANDARD HOOK BEND AND  
HOOKED DEVELOPMENT LENGTH DETAILS IN MASONRY DETAIL

SCALE: NTS

2

STRAIGHT DEVELOPMENT / LAP SPLICE LENGTH SCHEDULE (IN INCHES)											
MIN MASONRY COMPRESSIVE STRENGTH (f <sub>m</sub> =2500 PSI)	MINIMUM ( CC, CS) IN INCHES										
	1.5	2	2.5	3	3.5	4	4.5	5	5.5	6	
BAR SIZE / BAR DIAMETER (db)											
#4	0.500	30	24	24	24	24	24	24	24	24	24
#5	0.625	45	35	30	30	30	30	30	30	30	30
#6	0.750	54	54	51	43	37	36	36	36	36	36
#7	0.875	63	63	63	58	50	44	42	42	42	42
#8	1.000	72	72	72	72	72	66	59	53	48	48
#9	1.128	82	82	82	82	82	82	74	67	61	56

NOTES:

- CC INDICATES CLEAR COVER.
- CS INDICATES BAR CLEAR SPACING.
- db INDICATES BAR DIAMETER.
- INDICATES LENGTHS GOVERNED BY 9db.
- ALL LENGTHS INDICATED ARE IN INCHES.
- 72 db MAX, 48db MIN.
- IF ACTUAL CC (OR CS) FALLS BETWEEN THAT SHOWN IN SCHEDULE, DEVELOPMENT SPLICE LENGTH TO BE BASED ON SMALLER CC (OR CS)

TYPICAL REINF STRAIGHT DEVELOPMENT LENGTH/LAP SPLICE LENGTH SCHEDULE FOR MASONRY

SCALE: NTS

1

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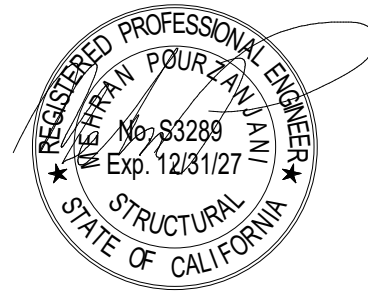
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TYPICAL CMU DETAILS

FIRE STATION 46

MISSION VILLAGE  
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VALENCIA, CALIFORNIA



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WRITTEN DIMENSIONS ON THESE DRAWINGS SHALL HAVE PRECEDENCE OVER  
SCALED DIMENSIONS. CONTRACTORS SHALL VERIFY AND BE RESPONSIBLE  
FOR ALL DIMENSIONS AND CONDITIONS ON THE JOB AND THIS OFFICE MUST  
BE NOTIFIED OF ANY VARIATIONS FROM THE DIMENSIONS AND CONDITIONS  
SHOWN IN THESE DRAWINGS. SHOP DETAILS MUST BE SUBMITTED TO THIS  
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10/20/2025

Date Issue Date

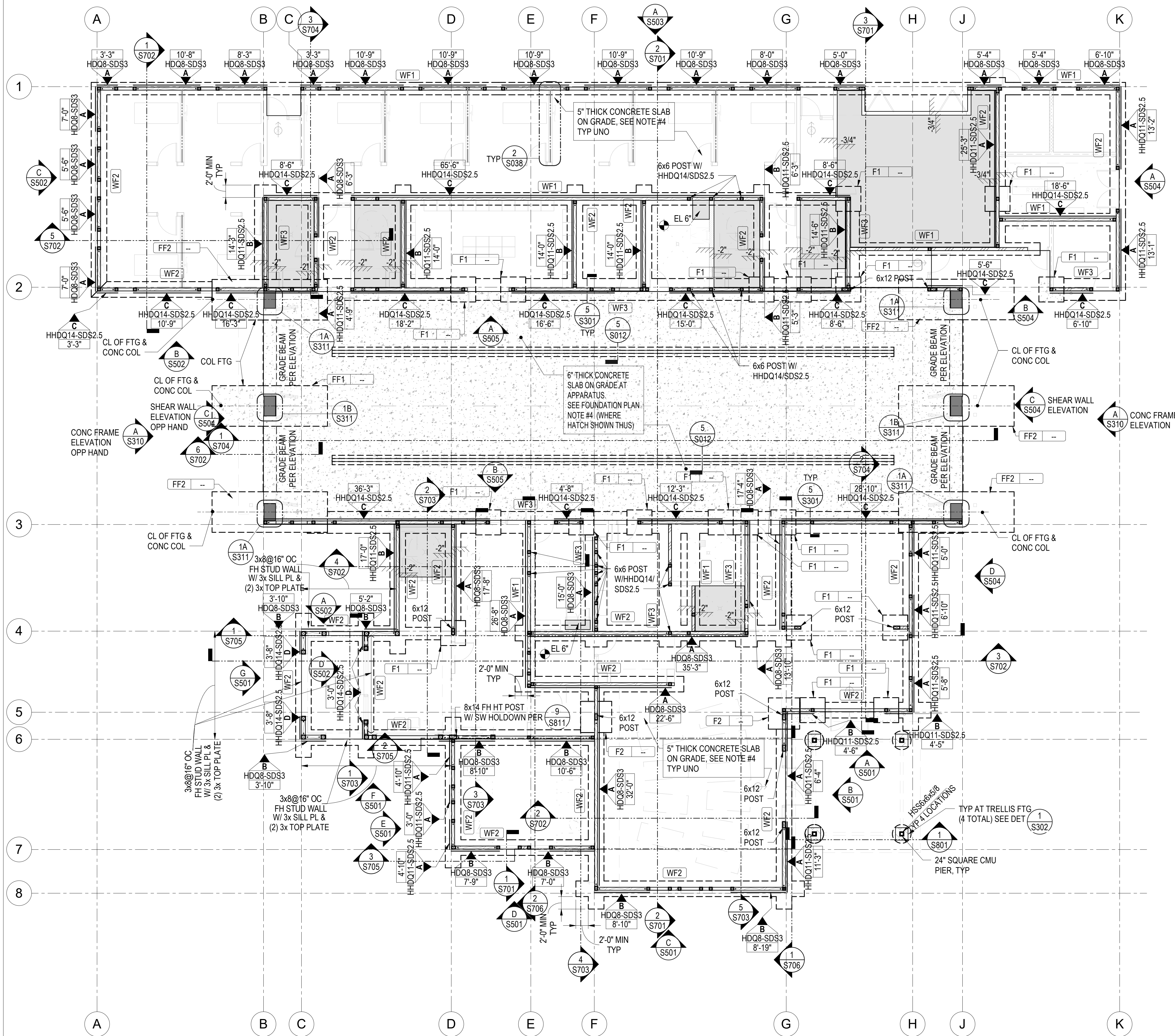
Drawn

Checked

Scale AS NOTED

Job No. Project Number

S021



### FOUNDATION PLAN NOTES

- FOR GENERAL NOTES SEE S0.0 SERIES AND TYPICAL DETAILS SEE S0 SERIES SHEETS.
- VERIFY ALL DIMENSIONS PRIOR TO START OF WORK. SEE ARCHITECTURAL DRAWINGS FOR REMAINDER OF DIMENSIONS NOT SHOWN ON THIS PLAN.
- SEE ARCHITECTURAL DRAWINGS FOR FLOOR ELEVATIONS, DEPRESSIONS, SLOPES, OPENINGS, CURBS, DRAINS, SLAB EDGE LOCATIONS, ETC., AND FOR WALL OVERALL DIMENSIONS, LOCATIONS OF OPENINGS, ETC., NOT INDICATED ON STRUCTURAL DRAWINGS.
- TYPICAL SLAB ON GRADE SHALL BE AS FOLLOWS UNLESS NOTED OTHERWISE:
  - 5" CONCRETE SLAB W/ #4@18"OC EACH WAY AT CENTER OF SLAB OVER
  - SLAB SHALL BE PLACED OVER 20 MIL VAPOR BARRIER UNDERLAID AND OVER 4 INCHES OF CLEAN COARSE SAND AND OVERLAID BY 2 INCHES OF CLEAN COARSE SAND, SEE SOIL REPORT.
- INDICATES 6" THICK CONCRETE SLAB ON GRADE
  - PROVIDE #4@12" EW AT MID-HEIGHT OF SLAB.
  - SLAB SHALL BE PLACED OVER 20 MIL VAPOR BARRIER UNDRALAIAD AND OVERLAID BY 4 INCHES OF CLEAN COARSE SAND AND OVERLAID BY 2 INCHES OF CLEAN COARSE SAND, SEE SOIL REPORT

- PROVIDE CONSTRUCTION JOINTS AND WEAKENED PLANE JOINTS (INDICATED AS CJ AND PW, RESPECTIVELY) IN SLAB ON GRADE AS SHOWN ON DETAIL (LOCATE PER ARCHITECTURAL DRAWINGS).
- INDICATES SHEAR WALL MARK FOR PLYWOOD SHEAR WALLS:
  - INDICATES SHEAR WALL TYPE PER SCHEDULE
  - INDICATES HOLDOWN TYPE EACH END OF SHEAR WALL PANEL SEE SCHEDULE (S309) FOR HD HOLDOWNS. OMIT HOLDOWNS WHERE HSS COL OCCURS AT END OF WALL.
  - INDICATES SHEAR WALL DESIGN LENGTH
- INDICATES HOLDOWN LOCATION. PROVIDE POST FOR HOLDOWN PER (S3039)
- TYPICAL WOOD STUDS SHALL BE AS FOLLOWS, UNLESS NOTED OTHERWISE ON PLAN
  - SHEAR WALL: 2x6@16" AT ALL EXTERIOR WALL. UNO 2x6 @16" AT INTERIOR WALL UNO
  - EXTERIOR WALL (EXCLUDING THE APPARATUS RESERVE BUILDING): 2x6@16" UNO
  - EXTERIOR WALL FOR APPARATUS RESERVE BUILDING: 2x8" @16" OC
  - INTERIOR BEARING WALL: 2x6@16" AND LARGER AS INDICATED BY ARCH
  - INTERIOR NON-BEARING WALL: 2x4 @16" FOR HEIGHTS UP TO 10' OR 2x6@16" AND LARGER AS INDICATED BY ARCH

- INDICATES INTERIOR BEARING WALL, SEE DETAILS ON SHEETS S021 & S022
- ALL NAILS ARE COMMON NAILS.
- FOR CONTINUOUS FOOTING SEE PLAN AND DETAILS ON SHEET S301
- WOOD WALL INTERSECTION CONNECTION SEE DETAILS (S3038) AT NON-SHEAR WALL & (S3038) AT SHEAR WALL.

### FOUNDATION PLAN LEGEND

- WF# INDICATES CONTINUOUS FOOTING SIZE. SEE PLAN (S301) FOR REINFORCING AND INFORMATION.
- FX X'-XX" INDICATES SPREAD FOOTING MARK SEE FOOTING SCHEDULE (S301)
- INDICATES TOP OF FOOTING ELEVATION SHALL BE MIN -1'-0" FROM TOP OF FINISHED GRADE OR LOWEST ADJACENT GRADE

ALL HARDWARE IS BY "SIMPSON" TYPICAL OR APPROVED EQUAL.

- OR POST INDICATES POST SHALL BE AS FOLLOWS:
  - 4x4 WOOD POST AT 4" INTERIOR STUD WALLS, UNO ON PLAN
  - 6x6 WOOD POST AT 6" EXTERIOR & INTERIOR STUD WALLS, UNO ON PLAN WHERE WOOD POST ARE ALSO USED AS SHEAR WALL HOLDOWN SEE DETAIL (S3039) & (S3038) AND USE THE LARGER POST
- INDICATES SLOPED SLAB ON GRADE REFER TO ARCH DWGS
- S- - - S INDICATES STEP FOOTING, SEE DETAIL (S302)
- INDICATES CMU WALL, SEE DETAILS ON SHEETS S021 & S021
- INDICATES RAISED OR DEPRESSED SLAB. SEE DETAILS COORDINATE W/ MECH/ELECT DWGS/ARCH A1.0 (S3011) OR (S3013)
- GB INDICATES GRADE BEAM, SEE DETAIL (S311)
- INDICATES PARTIAL HIGH DORM ROOM WALL PER SEE ARCH'L FOR LOCATION (S3038)

SHEAR WALL ELEVATION ON GRID 6

SCALE: 1/8" = 1'-0"

1

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FOUNDATION PLAN

FIRE STATION 46

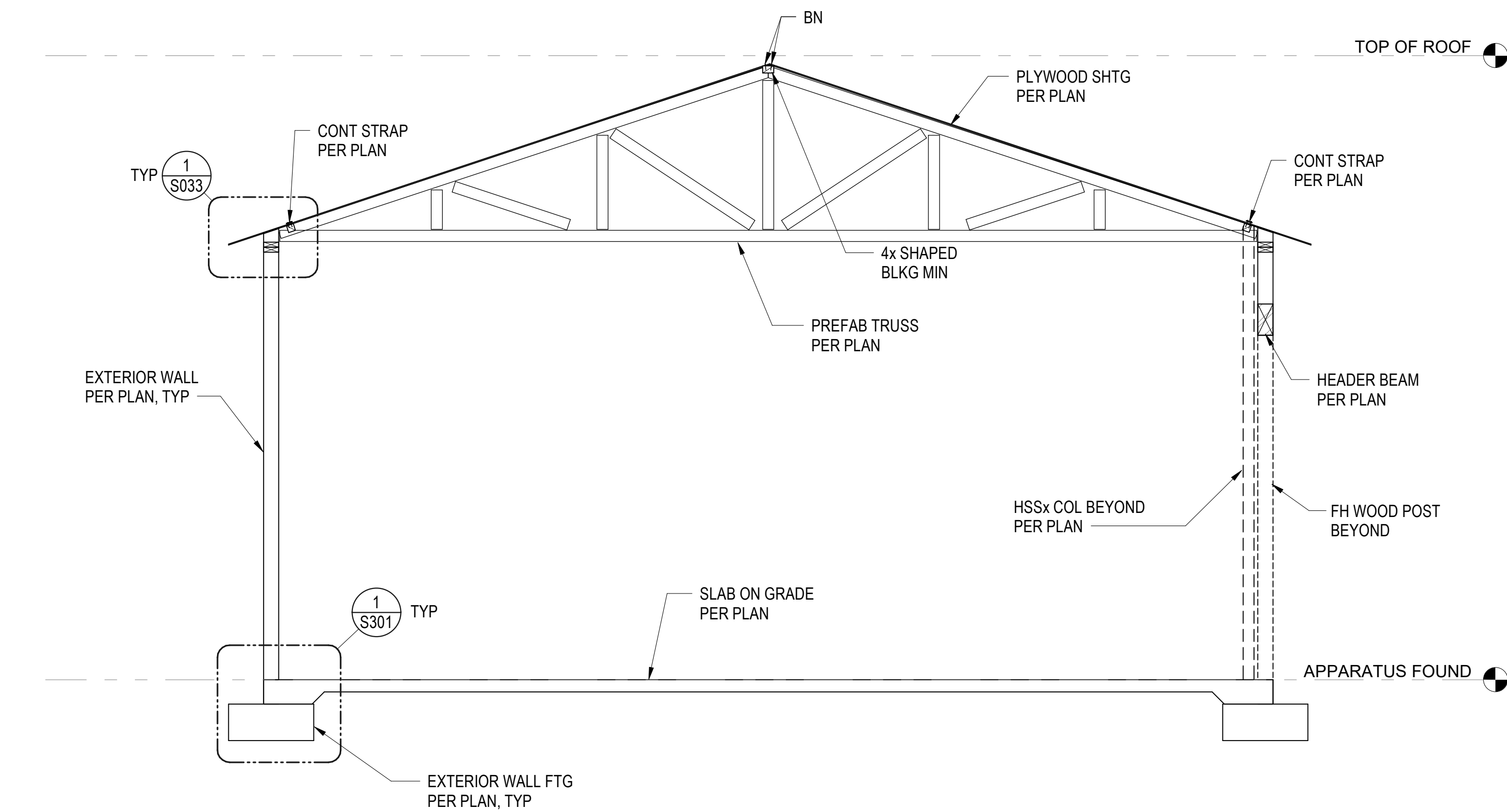
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Date	Issue Date
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Scale	AS NOTED
Job No.	Project Number

S210

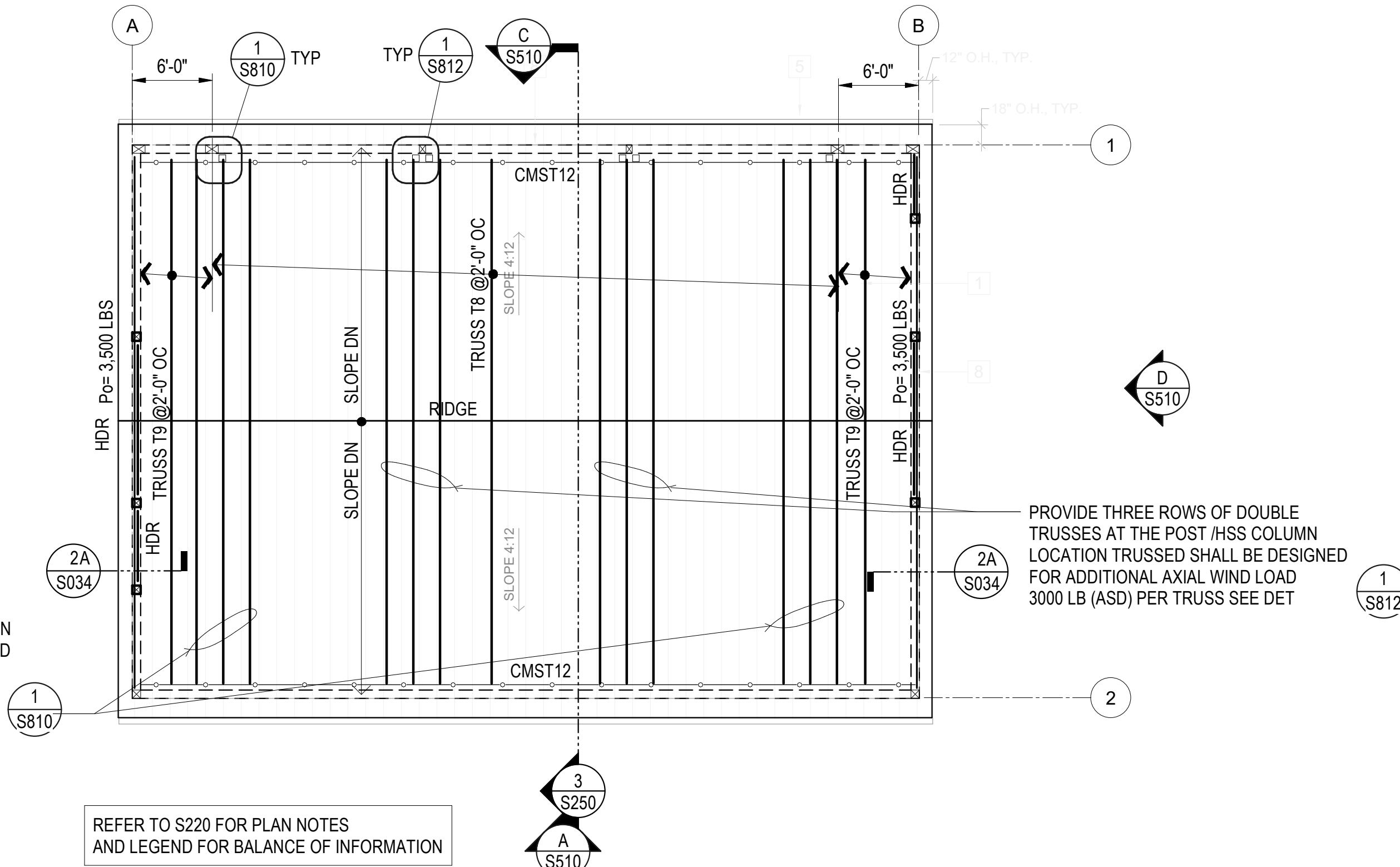


RESERVE APPARATUS SECTION

SCALE: 1/4" = 1'-0"

3

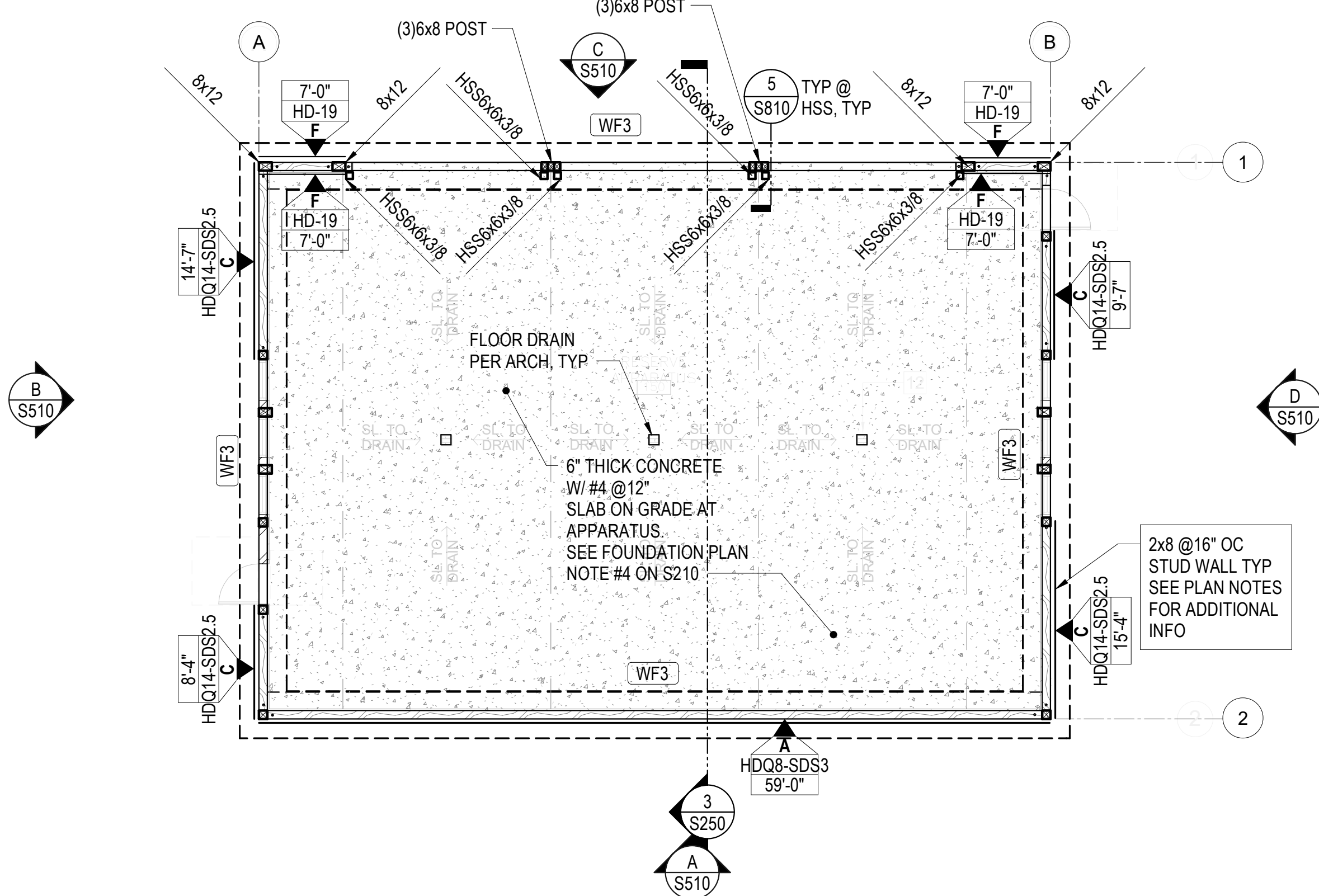
PROVIDE THREE ROWS OF DOUBLE TRUSSES AT THE POST /HSS COLUMN LOCATION TRUSSES SHALL BE DESIGNED FOR ADDITIONAL AXIAL WIND LOAD 1500 LB (ASD) PER TRUSS SEE DETAIL



RESERVE APPARATUS ROOF FRAMING PLAN

SCALE: 1/8" = 1'-0"

2



RESERVE APPARATUS FOUNDATION PLAN

SCALE: 1/8" = 1'-0"

1

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RESERVE APPARATUS  
PLANS AND SECTION

FIRE STATION 46

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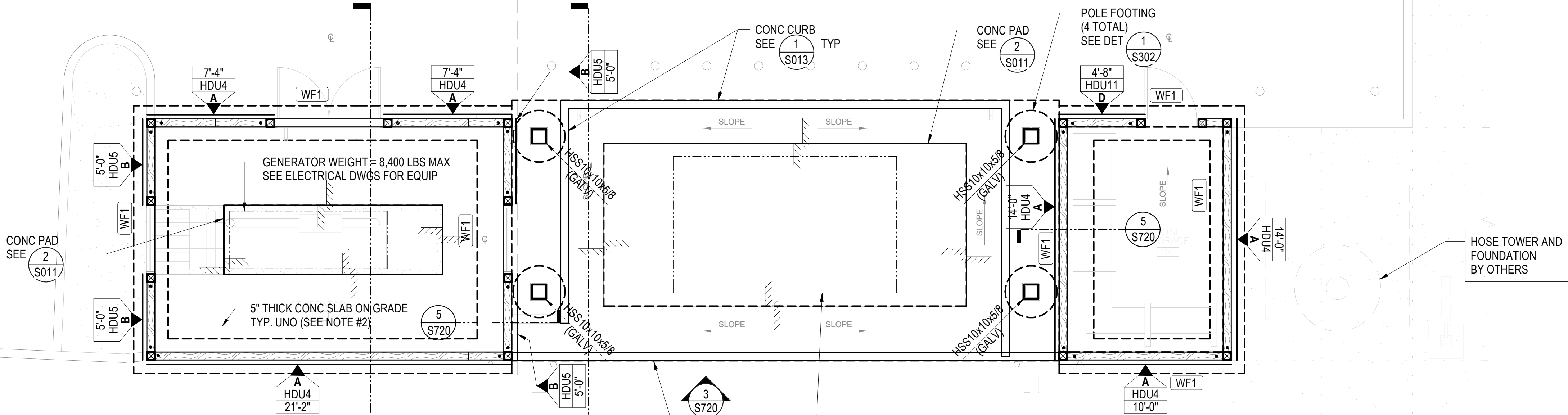


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Job No.


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AS NOTED  
Project Number

S250



ALL STRUCTURAL STEEL AND MISCELLANEOUS METALS EXPOSED TO WEATHER SHALL BE HOT DIPPED GALVANIZED AFTER FABRICATION UNLESS NOTED OTHERWISE ON ARCHITECTURAL DRAWINGS. ALL FIELD WELDING SHALL BE PAINTED WITH WEATHER PROOF PAINT PER THE SPECS AND ARCH REQUIREMENTS.

- NOTES:**
1. SEE FOUNDATION NOTES ON SHEET S210
  2. SEE NOTE #4 ON S210 FOR SLAB ON GRADE

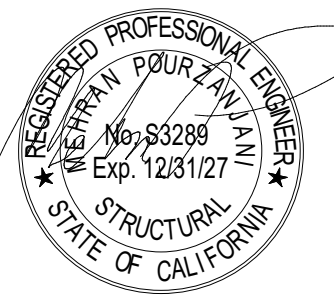
- CURBS:**
1. SEE ARCH'L DWGS FOR
    - a. LOCATION OF ALL CURBS.
    - b. FOR SIZE AND HEIGHT OF CURBS.
  2. SEE DETAIL  FOR CURB REINF.

## FUEL DISPENSING AREA FOUNDATION PLAN

SCALE: 1/4" = 1'-0'

# S260

Date	Issue Date
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Checked	
Scale	AS NOTED
Job. No.	Project Number

[illegible]

**FUEL AREA BUILDING AND  
CANOPY PLANS**

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MISSION VILLAGE  
COUNTY OF LOS ANGELES FIRE DEPARTMENT  
VALENCIA, CALIFORNIA



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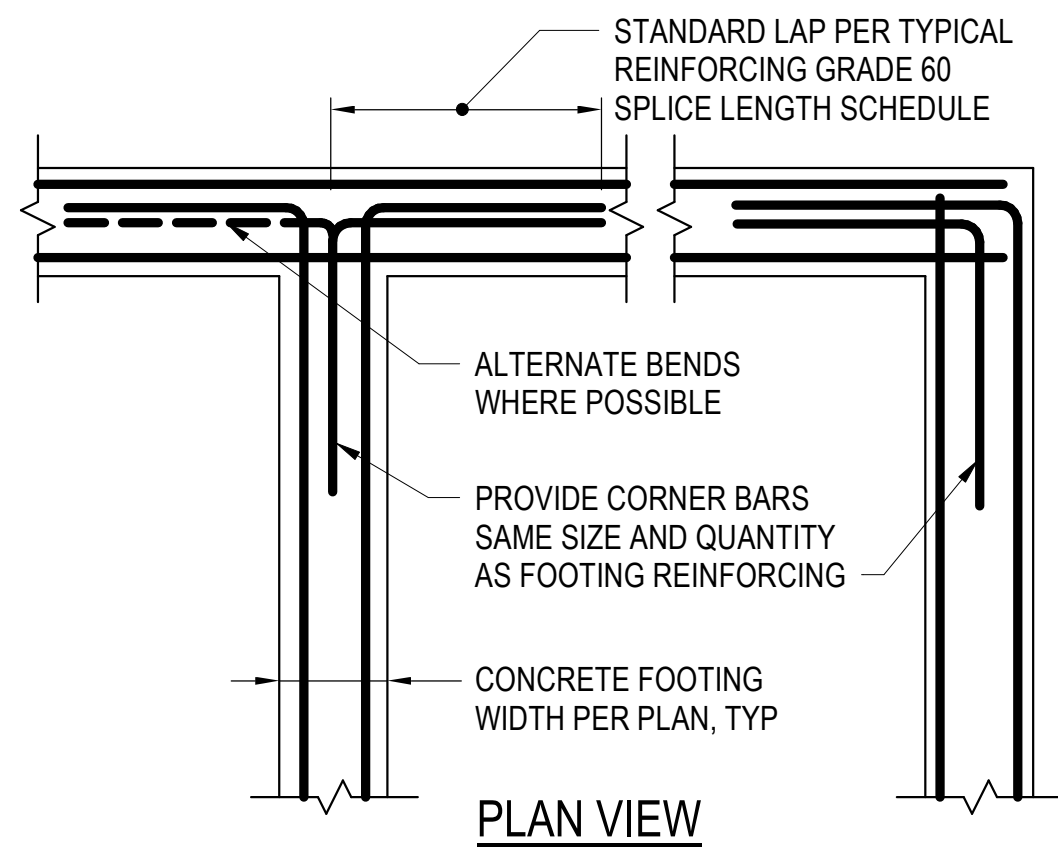
PLAN CHECK SUBMITTAL - October 31 2025

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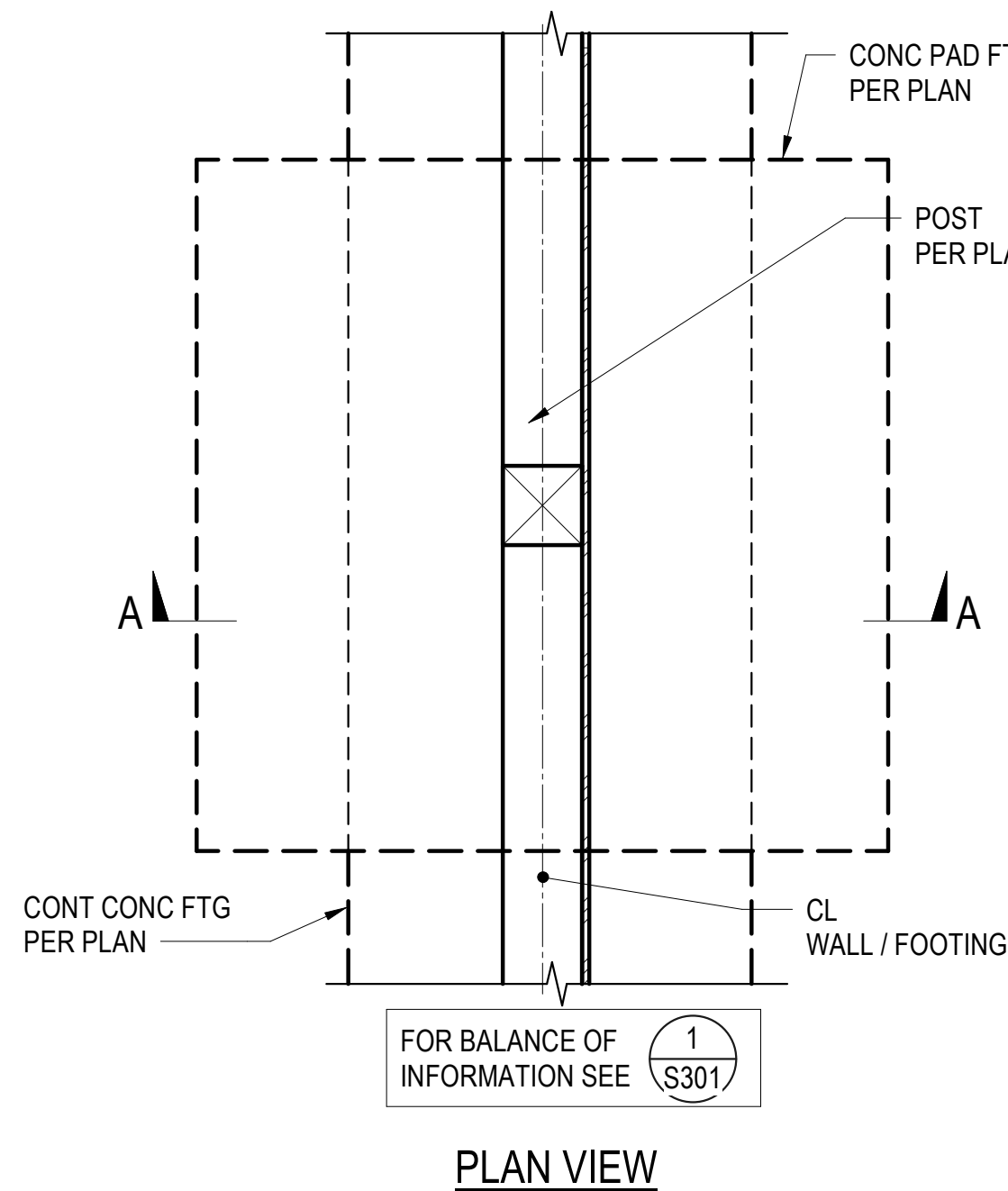
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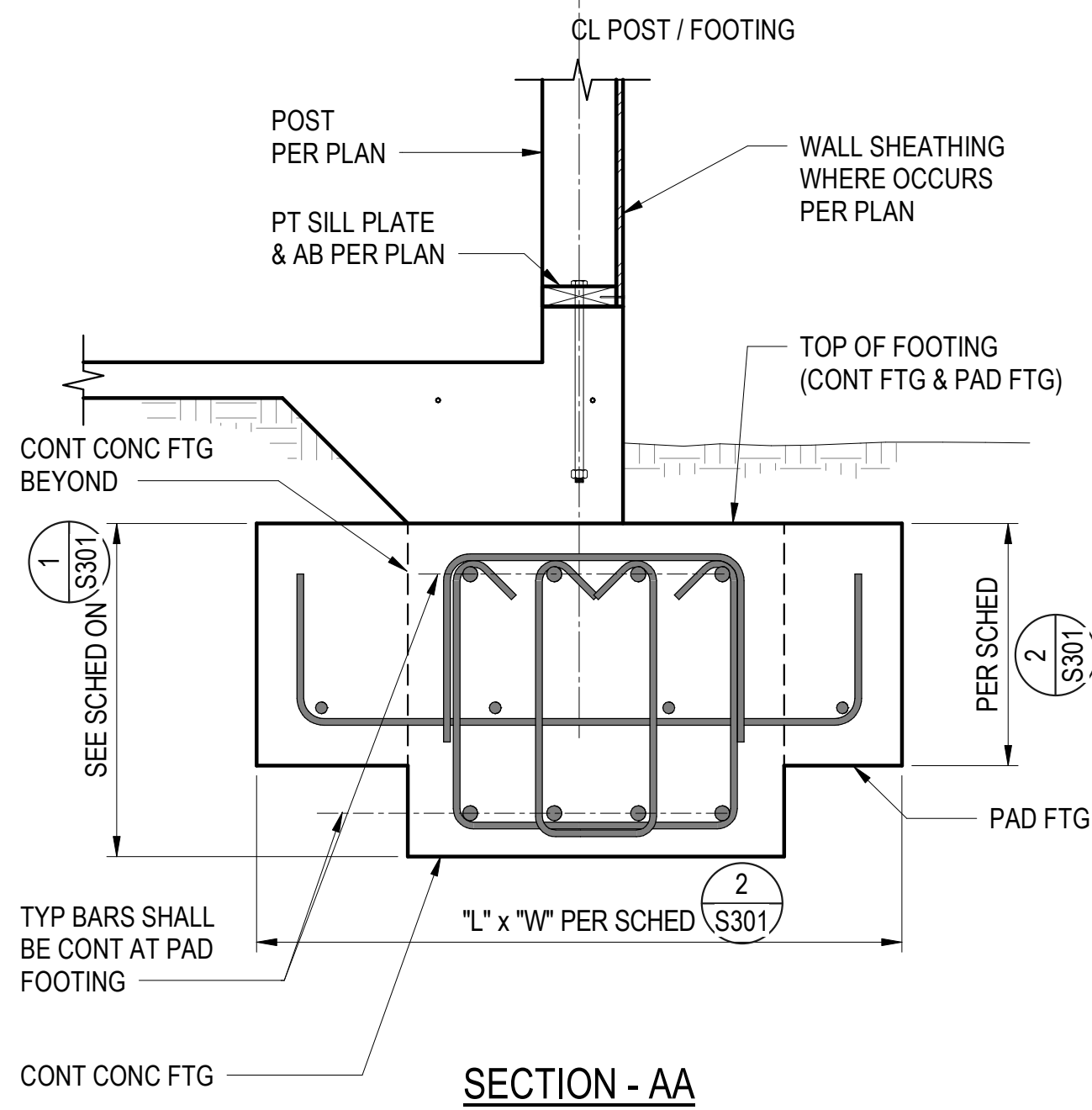
TYPICAL FOOTING REINFORCING AT CORNERS AND INTERSECTIONS DET

SCALE: NTS

7



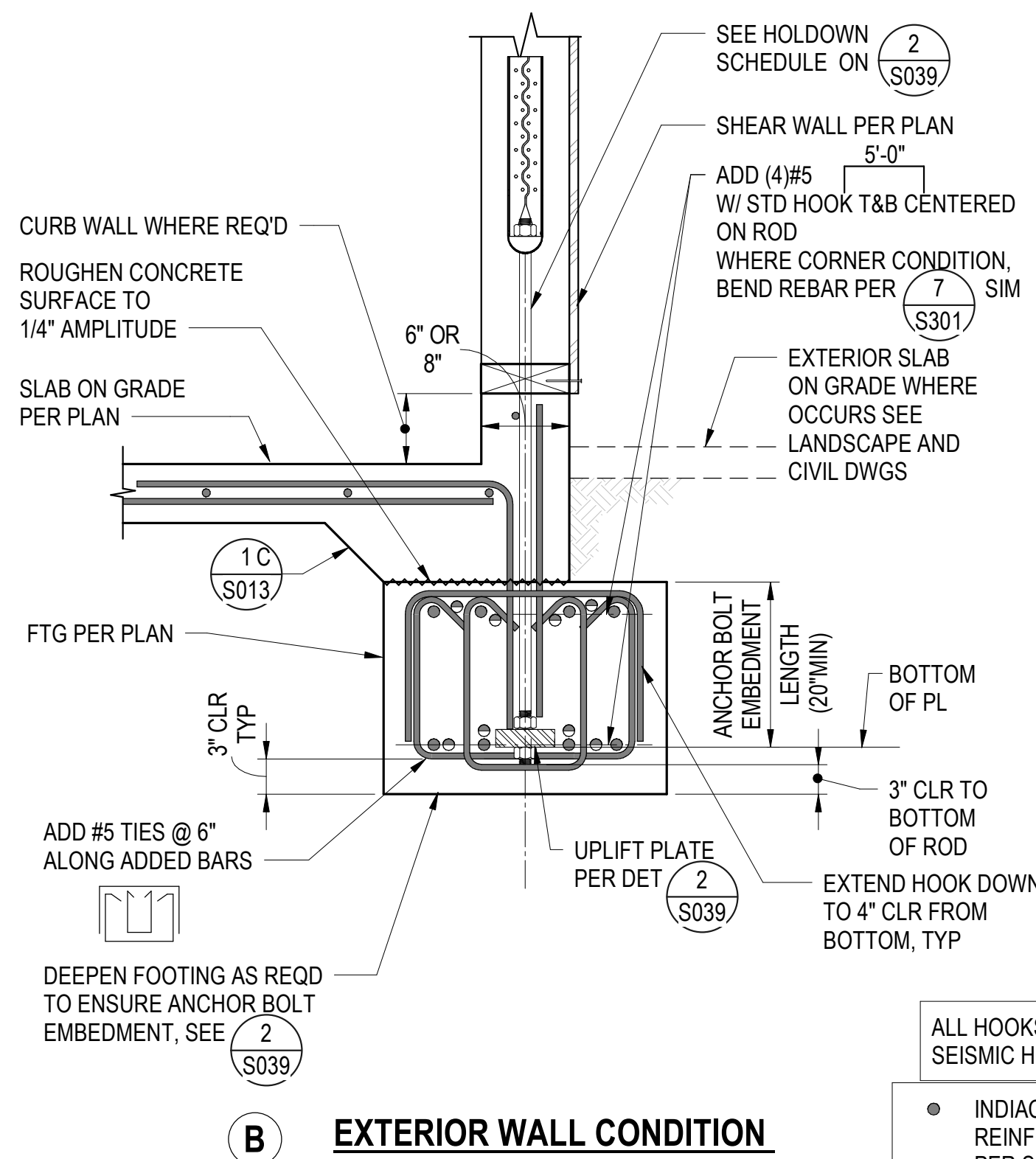
PLAN VIEW



TYPICAL PAD FOOTING AT CONTINUOUS WALL FOOTING

SCALE: NTS

6



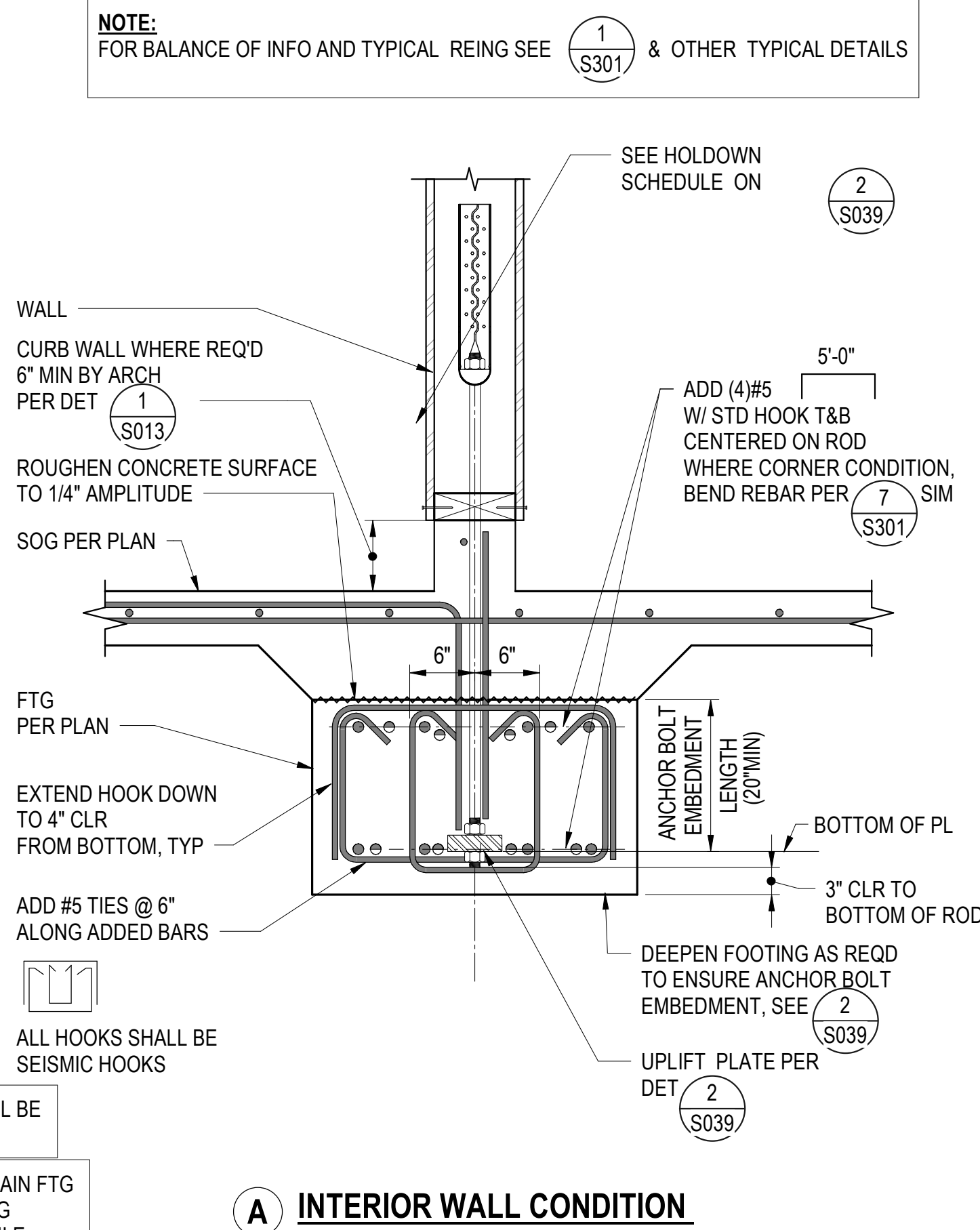
EXTERIOR WALL CONDITION

- ALL HOOKS SHALL BE SEISMIC HOOKS
- INDICATES MAIN FTG REINFORCING PER SCHEDULE.
- INDICATES ADDED REINFORCING.

TYPICAL SHEAR WALL HOLD DOWN ANCHORAGE WALL FTG DETAIL

SCALE: NTS

3

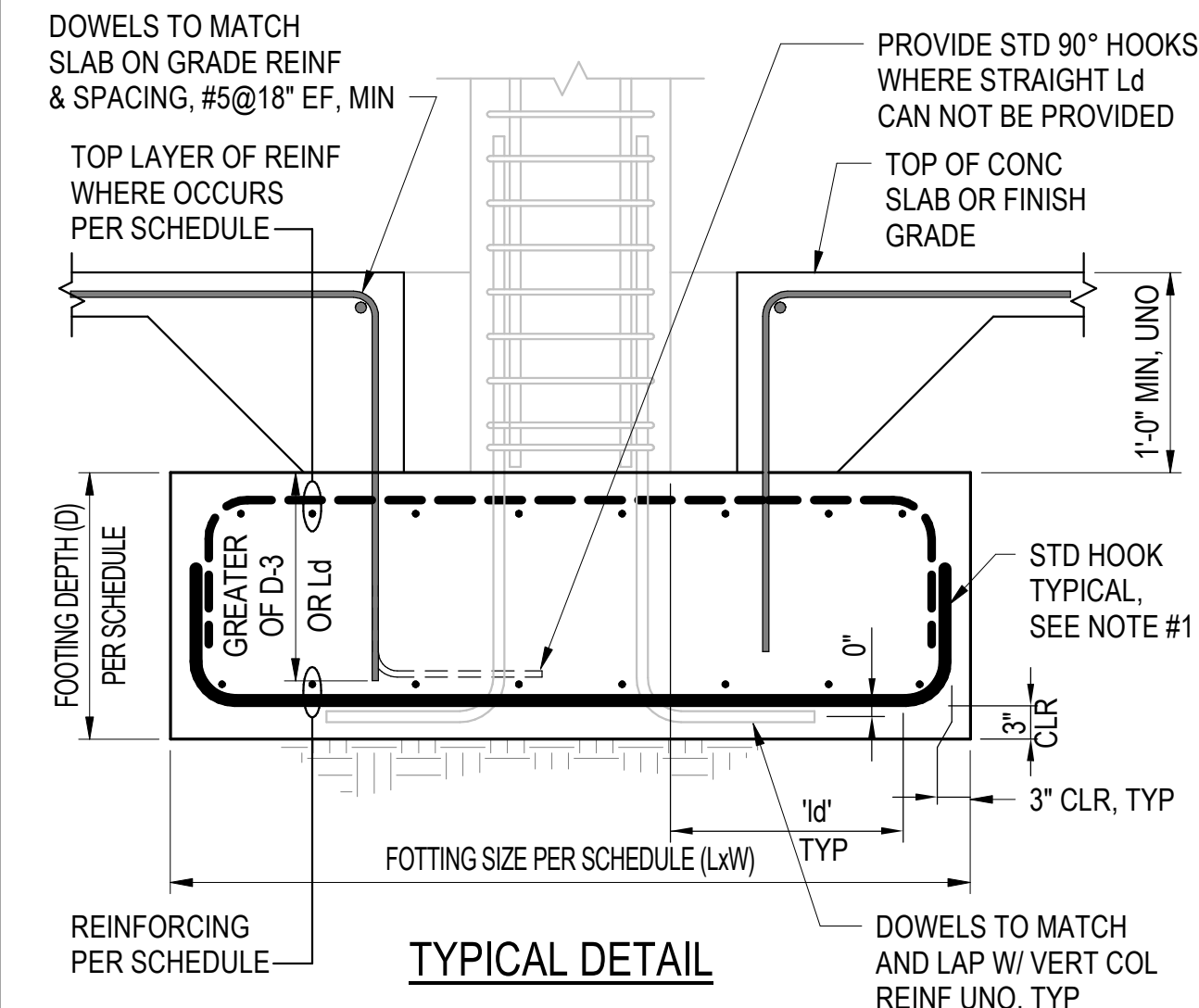


INTERIOR WALL CONDITION

TYPICAL SHEAR WALL HOLD DOWN ANCHORAGE WALL FTG DETAIL

SCALE: NTS

3



TYPICAL DETAIL

SPREAD FOOTING SCHEDULE

MARK	DEPTH (D)	SIZE (LxW)	REINFORCING				REMARKS
			BOTTOM		TOP		
			SHORT	LONG	SHORT	LONG	
F1	18"	4'-0"x4'-0"	(4)#6(B) EW	(4)#6(B) EW	-	-	
F2	18"	5'-0"x5'-0"	(6)#6(B) EW	(6)#6(B) EW	-	-	
FF1	36"	18'-0"x6'-6"	#8@12"	#8@12"	#8@12"	#8@12"	
FF2	36"	12'-0"x6'-6"	#8@12"	#8@12"	#8@12"	#8@12"	

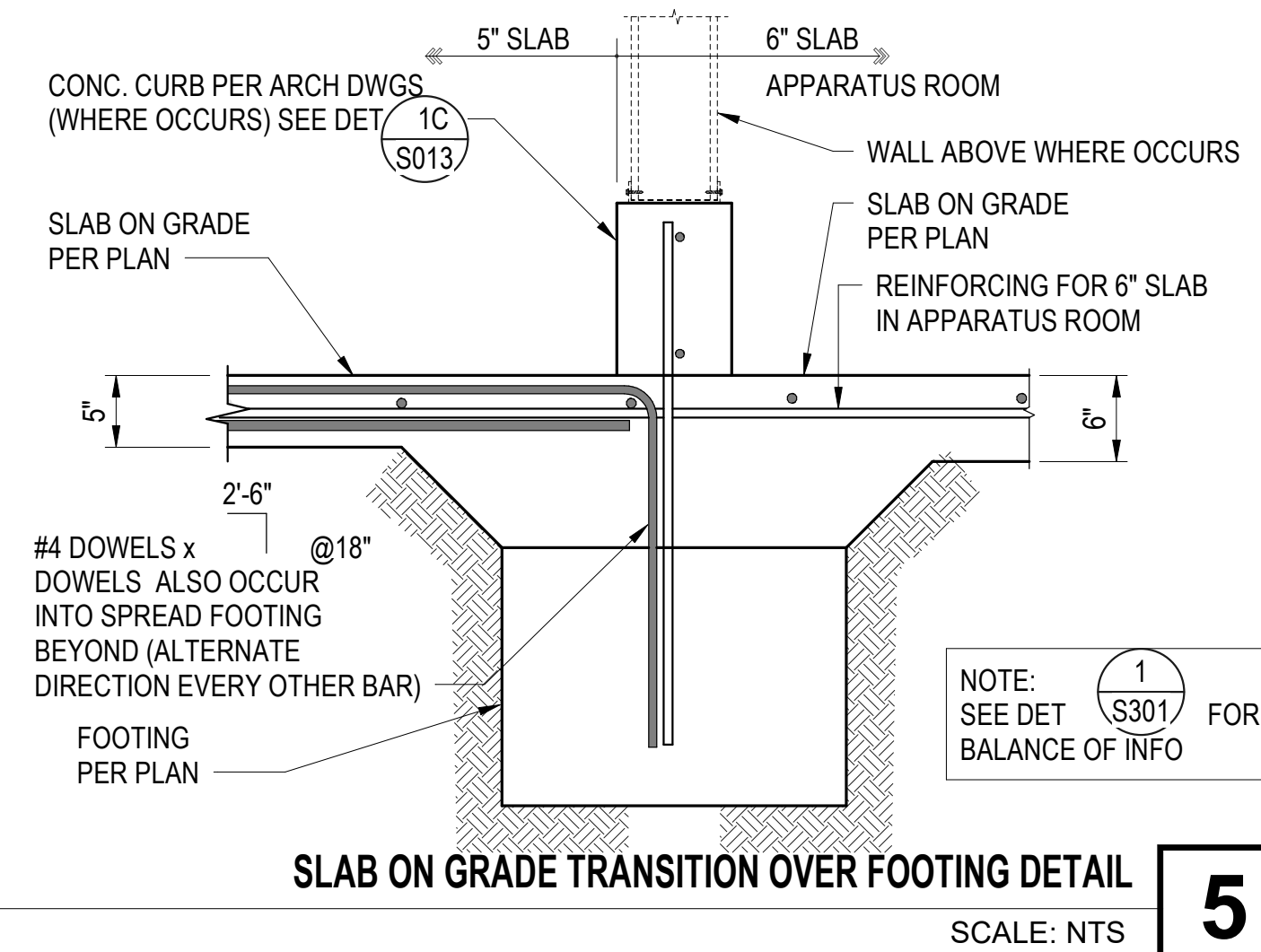
NOTES:

- "ld"=STRAIGHT DEVELOPMENT LENGTH PER "TYPICAL STRAIGHT AND HOOKED DEVELOPMENT LENGTH SCHEDULE".
- PROVIDE STANDARD HOOK WHERE A STRAIGHT BAR WITH "ld" CANNOT BE PROVIDED.

TYPICAL SPREAD FOOTING SCHEDULE AND DETAIL

SCALE: NTS

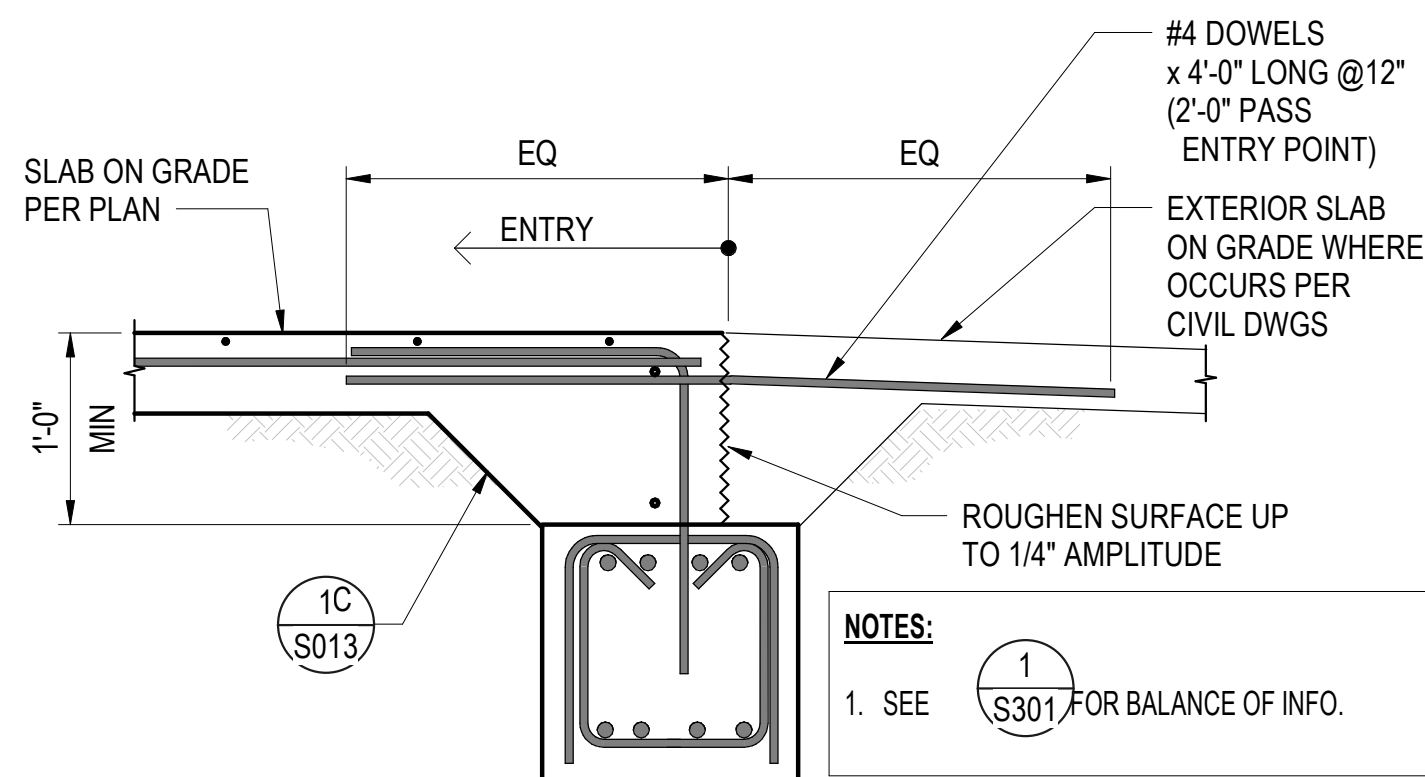
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SLAB ON GRADE TRANSITION OVER FOOTING DETAIL

SCALE: NTS

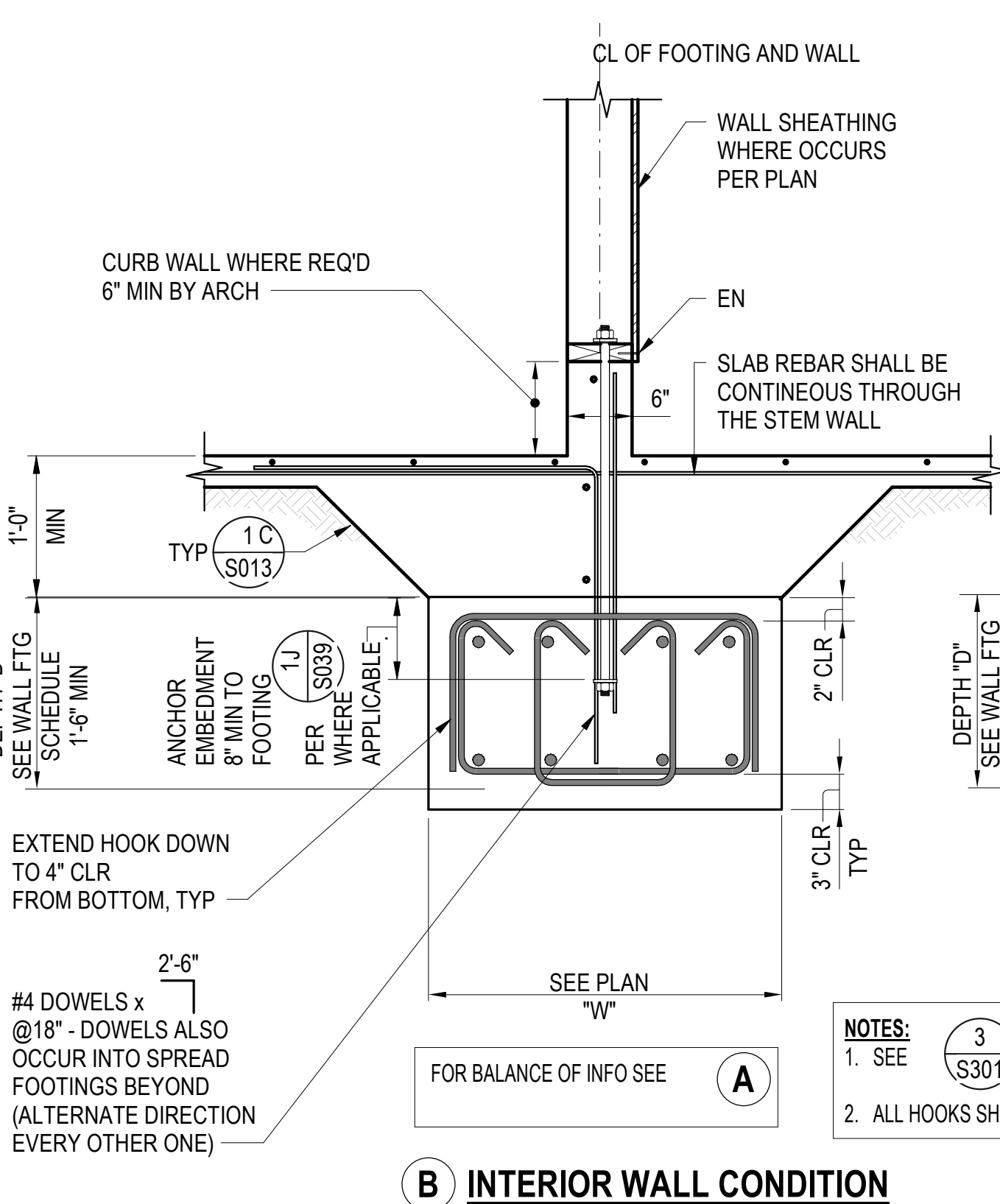
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TYPICAL PERIMETER FOOTING AT ENTRY

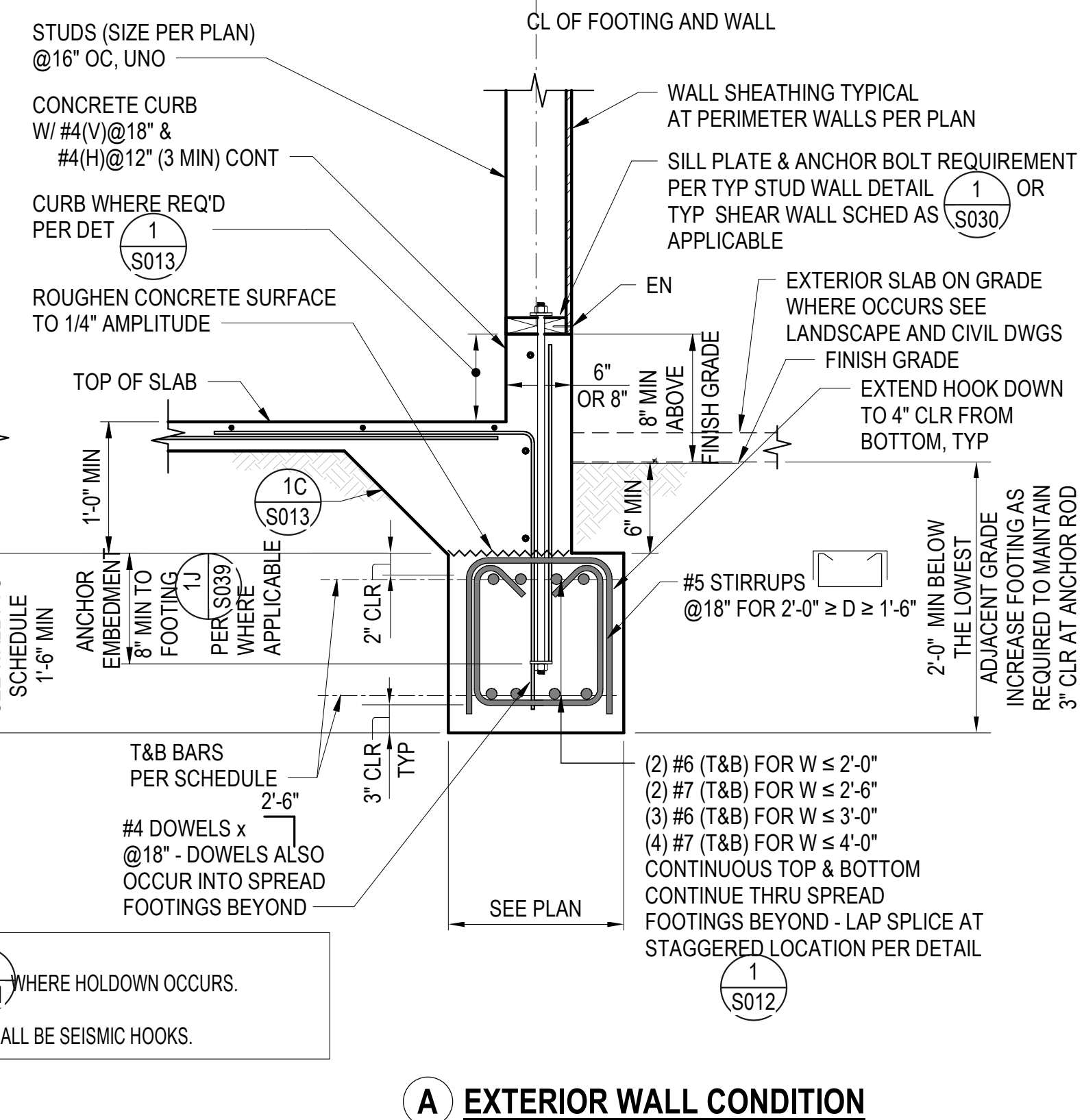
SCALE: NTS

4



INTERIOR WALL CONDITION

WALL FOOTING SCHEDULE			
MARK	DEPTH (D)	WIDTH (W)	REMARKS
WF1	18"	2'-0"	SEE DET
WF2	18"	3'-0"	SEE DET
WF3	18"	3'-6"	SEE DET



EXTERIOR WALL CONDITION

TYPICAL CONTINUOUS BEARING OR SHEAR WALL FOOTING DETAIL

SCALE: NTS

1

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CONCRETE FOUNDATION  
DETAILS

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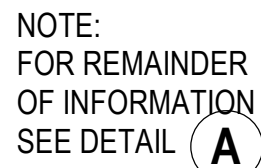
S301



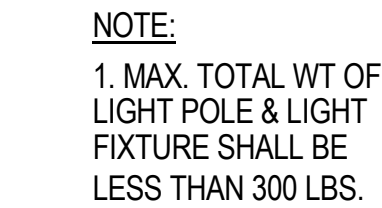
SCALE: NTS



SCALE: NTS



SCALE: NTS



**CONDITION B**  
**USE AT LIGHT POLE ONLY**



**NOTE:**  
ALL EXPOSED STEEL ELEMENTS INCLUDING BOLTS, PLATES AND POSTS SHALL BE HOT-DIP GALVANIZED

SCALE: NTS